

SHRIMPTON & LIPINSKI

**MAYFIELD**  
**RESIDENTIAL SUBDIVISION STAGE 2**  
**WESTMORLAND RISE, RAUKAWA DRIVE**  
**BETHLEHEM**

**REPORT ON SUBDIVISION EARTHWORKS**  
**& RECOMMENDATIONS FOR BUILDING**

Our Ref: 16530  
May 2003

**S&L CONSULTANTS LTD - SURVEYORS - ENGINEERS - PLANNERS**

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## **1.0 Introduction**

The earthworks, roading construction and services installation were completed on 30 April 2003 for Stage 2 of the Mayfield Subdivision in Bethlehem North. 54 residential lots were created in an extension of Westmorland Rise from the Stage 1 area and the construction of the long cul de sac of Raukawa Drive.

The locations and sizes of the 54 lots created are shown on DP 320267 (seven sheets). Lots 48, 89, 101, 108, 196 and 197 contain existing houses. Lots 96 and 196 are original large properties which have been altered slightly by boundary adjustment so that the new residential lots around the cul de sac head of Raukawa Drive can be created.

This report describes the earthworks undertaken in the formation of this stage of subdivision including the relevant standards adopted for the placement of filling to support residential buildings and recommendations for building including building restrictions where relevant.

During the report reference is made to drawings 16530-51 and 52 which are included in Appendix I to this report. These drawings show the relevant road and lot locations, areas of cut and filling and subsoil and fill compaction test locations.

## **2.0 Scope of Earthworks**

The earthworks undertaken in the Stage 2 area comprised:

- The formation of the road subgrade in cut from Lots 79 and 107 northwards with the areas of cut extending into the front areas of Lots 76 to 79 and encompassing the total areas of Lots 65 to 69 and 71 to 73 inclusive. Some of the cut soils were used for localised filling in Stage 2 but most were transported to future development areas on Westmorland Rise to the north of Stage 2. The maximum depth of cut made was 3 metres on the road subgrade opposite Lots 68 and 117. Elsewhere depths of cut were generally not more than 1.0 to 1.5 metres.
- Minor areas of cut along the road frontage of Lots 99 to 103 and within the rear areas of Lots 104 and 107 and extending into Lot 105.
- The placement of filling to the standards contained in NZS 4431:1989 and the Tauranga District Council Code of Practice for Development:
  - At the cul de sac head of Raukawa Drive.
  - To infill a depression left on Lot 83 where a swimming pool was removed.
  - To infill a depression left where a tennis court was removed on Lots 74 and 78 (up to 1.5 metres deep).
  - To raise the general ground levels on Lots 111, 112 and 116 to 121 (up to 0.9 metres deep).
  - To infill a silt pond required during the earthworks, on Lots 113 to 115 (up to 1.0 metres deep).
  - In general localised recontouring behind retaining walls erected on the road boundaries of Lots 71 to 73.

The depths of filling shown on drawings 16530-51 and 52 were derived from surveyed contours of the finished surface taken on the completion of the earthworks for this stage compared with the topographical survey undertaken by Connell Wagner Ltd for Mayfield Ltd prior to the subdivision construction.

The earthworks for Stage 2 development were undertaken by A & R Partnership subcontracted to Higgins Contractors Bay of Plenty Ltd during the 2002-2003 earthworks season in compliance with consent 61698 issued by Environment BOP.

### **3.0 Pre Subdivision Investigations**

Prior to obtaining subdivision approval a geotechnical assessment of the development area was undertaken by Connell Wagner Ltd on behalf Mayfield Ltd. The Connell Wagner report is referenced 7073-SH-04 and is dated June 1999. A copy of this report is present on the District Council subdivision file.

Their investigation determined (as quoted from the Connell Wagner report with their lot numbering modified to the numbering as shown on DP 320267) that:

- (a) The soils over the higher ground generally comprise typical ash deposits for this region with approximately 3 to 4 metres of younger ash overlying older Hamilton ashes, which in turn overlie the Tauranga formation.
- (b) The soils underlying the low lying ground below R.L 2.0 comprise very stiff marine silts to varying depth (these conditions were not present in the Stage 2 area).
- (c) The topsoil varies in depth but is generally 0.1 to 0.5 metres in thickness comprising dark brown sandy silt.
- (d) The younger ashes consist of firm to stiff light brown sandy silts and silty sands. These soils are underlain by the older Hamilton ashes which often comprise a dark brown silty clay layer overlying a series of interbedded brown and orange silty clays, clay silts and sands. The underlying Tauranga formation mostly comprises a series of light grey or white pumiceous sand and silts.
- (e) In the area of Lots 111 to 121 horticultural filling was identified from test pits. This area was in an extension of the narrow increased gully (extending south west). The soils in this area are stiff and overlie natural ash deposits at a depth of 2 metres. The undrained shear strengths measured in the old filling exceed 100kPa but are generally less than 150 kPa.
- (f) Within the upland head of the incised gully (now following Raukawa Drive) the land had been broadened into natural contoured landform. Some past earthworks is expected in the areas on Lots 102 to 109, 82 to 87, and 73 to 80 with least thickness in the areas of Lots 84, 85, 86 and 102.

The Connell Wagner report recommended that where the past horticultural filling had been poorly compacted and overlaid original organic soils within the old gully floor the filling and organic soils should be removed and be replaced. This recommendation was made for the "northern one third" of the gully which is north of Lot 121 and within the future stage of Mayfield as an extension of Westmorland Rise. The report recommended in the "Central and Southern portions" of the old gully and drainage depressions where the filling is less than 1 metre thick and had been placed over natural silt soils after topsoil stripping, that the filling could remain in place after an on site review or partially or totalling removing it and re-compacting back in place.

Additional site investigations in the Stage 2 area were undertaken by S & L Consultants Ltd to determine the type and extent of the horticultural filling. These comprised the excavations of test pits numbered TP3 on Lot 120, TP4 on Lot 117, TP5 on Lot 11, TP6 on Lot 74, and TP7 in front of Lot 79. The pit locations are shown on drawings 16530-51 and 52 and the pit logs are contained in Appendix IV to this report.

The pre-subdivision investigations identified the presence of horticultural filling along the road alignments and within the lots to the east of the roads. In these areas the filling was either removed as cuts were made to the road subgrade or for the silt pond on Lots 113 to 115 or was examined when topsoil was stripped prior to placement of filling. It was found unnecessary to undertake the removal of the existing filling because soils strengths were seen to be uniformly high and there was no evidence of old topsoil being present under the filling (refer to the post subdivision test results described in Section 5.0 below).

The initial subdivision walkover inspection identified the presence of non structural filling present behind a wall constructed of tyres now present on Lot 109. The tyred wall and filling placed in a garden were part of the yard development around the house which is on part of adjacent Lot 108. No subdivision earthworks were undertaken in Lots 108 or 109 to modify the ground including the non structural fill or tyre wall. The extent of the non structural filling is shown on 16530-51.

#### **4.0 Earthworks Standards**

The performance specification required of the Contractor for the earthworks was based on the guidelines contained in NZS 4431:1989 "Code of Practice for Earthfill for Residential Development". Enforcement of the compaction requirements listed below satisfies the standards listed in Section 7 of NZS 4431.

Air voids percentage (as defined in NZS 4402: Part 1: 1980)

- Structural Fill - average value less than 10% (any 10 tests)
- Maximum single value 12%

Undrained shear strength (measured by insitu vane)

- Structural fill - average value not less than 150kPa (any 10 tests)
- Minimum single value 100kPa

The calculation of air voids percentage requires the input of a value for the specific gravity of the compacted soil. It was found that some of the soils used in the filling were derived from the pumiceous sands present at the lower depths of the younger ash soils. Because of the angularity of these soils compaction densities were found to be lower than when cohesive soils were worked and compacted to the same degree. Where pumiceous sands were identified in the samples taken at the test sites by the testing laboratory the permissible air voids percentage maximum value was increased to 15.

The earthworks were observed by an engineering technician from this office and compaction and strength control testing was undertaken by local IANZ accredited soil testing laboratories in Tauranga both on site and in the laboratory.

19 compaction tests were undertaken within the areas of filling at locations shown on 16530-51 and 52. The compaction test results are summarised in the tabulation contained in Appendix III. The tests were generally taken after 0.5 metres of filling in the deeper fills had been placed and 0.3 to 0.4 metres below the completed surface in other areas.

All test results fall within the acceptance criteria with most developed shear strengths being so high that the probe could not be pushed in or the readings exceeded the dial capacity.

## **5.0 Post Construction Testing**

Post construction handaugered boreholes were put down on each lot at locations shown on 16530-51 and 52. These boreholes were generally 1 metre deep and were intended to confirm ground bearing conditions for shallow building foundations that were identified in the pre-subdivision boreholes and pits and during observations of services trench cuts and exposed soils prior to the placement of topsoil and grassing.

As the boreholes were being drilled undrained shear strengths were recorded with a hand held shear vane pushed in advance of the auger.

Summary logs of these boreholes and the shear strengths recorded are contained in Appendix IV.

## **6.0 Summary & Recommendations**

### **6.1 Subdivision Construction Filling**

Structural filling as shown on drawings 16530-51 and 52 was placed in accordance with the methods and standards quoted in NZS 4431 under the supervision of S & L Consultants Ltd. Compaction testing on site confirmed that a high and uniform degree of compaction has been achieved suitable for the support of buildings. Post construction boreholes also confirmed this suitability.

Our statement in support of the suitability of the filled areas for the erection of buildings in terms of NZS 3604 is appended in Appendix II of this report.

Within areas of structural filling on which buildings may be erected, however, the possibility of variation of soil type and strength may exist away from our observation or compaction test locations. The normal inspection of foundation conditions during construction of buildings by competent tradesmen as described in NZS 3604 and by building certifiers should therefore be undertaken. If for any reason areas of low soil strength are found professional geotechnical advice should be sought.

## **6.2 Areas of Cut**

In areas of cut the subsoils present on these lots will be ash derivative soils typical of those found in the Tauranga area.

The varying depths of cut have however exposed a variety of different soils types immediately below the topsoil overlay. This is because the more recent ashes which extend below the original ground level and which comprise a stiff upper mantle of light brown friable silt overlying bands of yellow or light grey pumiceous sand have been partially or totally removed.

Post construction handaugered boreholes were put down on all lots at locations shown on 16530-51 and 52 and logs of the soils found in these boreholes are contained in Appendix IV.

Each soil type identified had varying undrained shear strengths or degrees of compaction. The tests showed that undrained shear strengths in the insitu soils are sufficient for the construction of shallow building foundations.

For all lots located in areas of cut the post construction boreholes indicate that ultimate ground bearing pressures for foundation design may be taken as 300 kPa in the limit state. This capacity meets the definition of "good ground" as defined in NZS 3604. In situ and as tested, the soils present in the cut areas are of adequate strength for bearing capacity of 300 kPa. However if they are disturbed or are found to be variable during construction, foundations detailed in accordance with NZS 3604 may have to be deepened or widened accordingly.

## **6.3 Areas of Undisturbed Ground**

Areas of ground not altered by the subdivision earthworks exist generally at the rear of lots away from the road carriageway, berms and some front yards to the lots. Pre and post subdivision investigations indicate that insitu soils or minor depths of horticultural filling would exist in the areas not modified during the subdivision construction. Tests taken during these investigations indicate that shallow building foundations can be constructed and that ultimate ground bearing pressures for foundation design may be taken as 300 kPa in the limit state. This capacity meets the definition of "good ground" as defined on NZS 3604.

As the subdivision area was previously developed as an orchard with some trees having been removed it is possible that near surface soils may have been disturbed during the removal of trees or possibly from root pruning along the orchard shelter that is still present along the eastern boundaries of Lots 111 and 121. Any area cleared for building by the removal of topsoil on the areas not modified by the subdivision earthworks as seen on 16530-51 and 52 should be inspected for evidence of such disturbance or possibly the presence of deeper topsoil or horticultural filling. Minor excavation with a spade could determine what depths of disturbed ground are present. If such disturbance or soft or loose or organic filling is found to extend deeper than the proposed building foundation geotechnical engineering advice should be sought.

If building development on Lot 109 is to utilise the area of the filling behind the rubber tyred wall additional site investigations should be undertaken to determine the extent and depth of this filling. The post subdivision investigation borehole on Lot 109 showed the presence of 0.4 metres of topsoil being present. As this lot was not modified by subdivision earthworks it is possible that such a topsoil depth may exist over the remainder of the lot or it may be thicker. The investigation to determine the stability of the non structural fill behind the tyres should also be extended to identify the topsoil depth and continuity of the natural ground found in the post construction borehole.

Houses exist on Lots 48, 89, 101, 108, 196 and 197. No specific tests have undertaken on these sites. It is assumed that adequate foundation conditions existed at the time of construction of these houses. Should extensions or modifications to these houses be contemplated in the future the building designer or owner should at least review any site records kept on the property file at the Tauranga District Council. If required additional site tests could be undertaken to confirm the presence of "good ground" in any redevelopment area on those lots.

No specific investigations or tests have been undertaken in large lots 96 and 196 because any further development would be subject to a separate consent in the future. Anecdotal information indicates that natural undisturbed ground exists on these properties.



#### 6.4 Land Stability

Most of the area on the lots contained in Stage 2 at Mayfield comprises near flat or gently sloping ground. In these areas no global stability issues exist that may restrict or prevent buildings being erected.

Restrictions due to possible localised stability issues are recommended on the following lots:

##### Lot 48

A house is present on this lot which was formerly Lot 48 DP 307712. A consent notice on the title for Lot 48 DP 307712 refers to advice given by Connell Wagner Ltd for any further development of this lot. This consent notice is to be carried forward to the new title of Lot 48 DP 320267. During the construction of Westmorland Rise timber retaining walls were constructed along the road boundary in accordance with a building consent issued by the Tauranga District Council on 21 March 2003. Geotechnical advice should be sought if it is intended to place additional filling or construct an accessway or building within 7 metres of these walls. Such advice should also be sought if it is intended to undertake similar development above the unretained batters along the driveway or the southern boundary or above the rock walls along the eastern boundary. A copy of the approved and constructed timber wall details is contained in Appendix I.

##### Lot 69

A cut and grassed batter face was formed on the northern boundary as part of the subdivision earthworks. A building restriction line is shown on DP 320267 and is intended to set buildings away from the batter. The clearance provided is to allow access for maintenance of the batter and to permit a retaining wall to be constructed if required along the length of any future building. If the wall is constructed with a building consent the restriction on building close to the retained batter may be waived.

##### Lots 71, 72 & 73

Unretained cut batters exist at the rear of Lots 72 and 73. It is unlikely that buildings would be erected close to these batters and so no formal building restrictions are necessary. The batter faces, while grassed, may still be vulnerable to long term erosion and a retaining wall or some form of erosion prevention facing may be required in the future to maintain long term stability. Retaining walls have been erected along the roadside boundaries of Lots 71, 72 and 73 at batters cut when the road was formed. A building consent was issued for the walls by the Tauranga District Council on 21 March 2003. The walls have been designed to resist a modest surcharge but not loadings imposed on the ground directly above from additional filling even behind retaining walls, and other buildings. No additional filling, retaining walls or other buildings should therefore be located closer than 3 metres from the top of these walls. A copy of the details of the timber walls that were approved and constructed is contained in Appendix I.

#### **6.5    Topsoil Thickness**

During the subdivision earthworks areas of cut or fill were initially stripped of topsoil and this was then replaced to depths of up to 300mm. Outside of the earthworks areas where the ground was not disturbed it is possible that topsoil depths may be deeper than 300mm where the topsoil depth was developed naturally or where it was deepened due to past horticultural activities. No guarantee is implied or given that the topsoil on any part of any lot is 300mm deep or less and it is recommended that future owners or builders check topsoil depths when preparing site development plans and cost schedules.

#### **7.0    *Professional Opinion***

Our statement in the format of Council's Code of Practice for Development (Form G2) that all lots are suitable for building is contained in Appendix II. This statement is accompanied by form G2A which summarises the information and recommendations within this report.

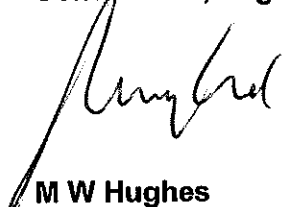
#### **8.0    *Applicability***

Recommendations contained in this document are based on data from borehole and pit data, observations of soil exposures, and test results. Inferences about the nature and continuity of subsoils away from these locations are made but cannot be guaranteed.

In all circumstances, if variations in the subsoils occur which differ from that described or assumed to exist the site should be inspected by an engineer suitably qualified to make an informed judgement and provide advice on appropriate improvement measures.

This report has been prepared specifically for the development at Stage 2 of the Mayfield Subdivision and no responsibility is accepted by S & L Consultants Ltd for the use of any part of this report for other development sites without their written approval.

**S & L Consultants Ltd**  
**Consultants, Engineers, Surveyors, Planners**



**M W Hughes**  
**Geotechnical Engineer**

**5 May 2003**

## **APPENDIX I**

**Drawings - Earthworks Completion Plans 16530-51, 52  
Deposited Plan DP 320267 (7 sheets)  
Retaining Wall Details 16530-41, 42**





- KEY**
- 736 Construction Compot Test Position
  - Post Construction B by S & L Consult
  - ⊕ 777 Pre subdivision Test by S & L Consult
  - ⊕ 779 Pre subdivision Borel by Gannett Wagner
  - 10 — Depth of Structural
  - 10 — Depth of Cut

Form 1 ZM Application

Project No.	NAME	DATE
111	Surveyed	
	Designed	SD 05/03
	Drawn	NH 05/03
	Checked	
	Approved	

REVISIONS

No.	Description
1	



**S & L CONSULTANTS**  
SURVEYORS - ENGINEERS - PLANNERS  
111 Commerce Road, Tauranga, New Zealand  
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Tel: 07 777-4005  
Fax: 07 777-4005  
Email: sl@slconsultants.co.nz

**TITLE**  
**MAYFIELD SUBDIVISION**  
**STAGE 2**  
**RAUKAWA DRIVE**  
**WESTMORLAND RISE**  
**COMPLETED EARTHWORK**  
**REFERENCE PLAN**

Copyright on this drawing is reserved

ORIGINAL SCALE	DATE
1:500 @ A1	05

DRAWING No. **16530-52**

Revisions

No.	Description
1	



**EXISTING LAND COVENANT**

PURPOSE	SHOWN	LOT	DOCUMENT
Height Restriction	F	82	H.912808
	G	198	
	H & I	81	

**MEMORANDUM OF EASEMENTS IN CROSS**

PURPOSE	SHOWN	SERVIENT	GRANTEE
Right to convey water	D & G	Lot 198	Tauranga District Council
Right to convey gas & liquids, right to transmit electricity & data & communications	D & G	Lot 198	Powerco - Ltd

**APPROVALS**

I hereby certify that this plan was approved by the Tauranga District Council pursuant to Section 223 of the Resource Management Act 1991 on the \_\_\_\_\_ day of \_\_\_\_\_ 2003 subject to the granting or reserving of the easements set out in the Memorandum hereon and subject to the amalgamation conditions set out hereon.

Authorised Officer sub 5311

Pursuant to Section 321(3)(c) Local Government Act 1974 the Tauranga District Council is satisfied that adequate access to Lots 81, 82, 83, 87, 88 & 196 hereon is provided over other land pursuant to a condition imposed under Section 220(1)(b) of the Resource Management Act 1991 and (iv) of the Resource Management Act 1991 and is satisfied that adequate access to Lot 96 is provided by way of a right of way and that subsection 321(1) of the Act shall not apply. Dated this \_\_\_\_\_ day of \_\_\_\_\_ 2003.

Authorised Officer

**MEMORANDUM OF EASEMENTS**

PURPOSE	SHOWN	SERVIENT	DOMTEN.
Right to convey water & gas, Right to transmit telecomm & electricity	A, B	Lot 197	Lots 89, 196
	B, C	Lot 197	Lot 96
	D, G	Lot 198	Lots 80, 81, 196
	E	Lot 199	Lots 87, 88

**CLASS 1 SURVEY**

Total Area 9.7077 ha

Comprised in C.T. 500707, 540469, 578139, 578140, 29971, 29978, 29994

**JOHN DAVID BARNES**

being a person entitled to practice as a licensed cadastral surveyor certify that the survey to which this document relates is accurate, and was undertaken by me or under my direction in accordance with the Cadastral Survey Act 2002 and the Surveyor General's Rules for Cadastral Survey 2002.

(b) This document is accurate and has been created in accordance with the Act and these Rules.

Sign: \_\_\_\_\_ Date: \_\_\_\_\_

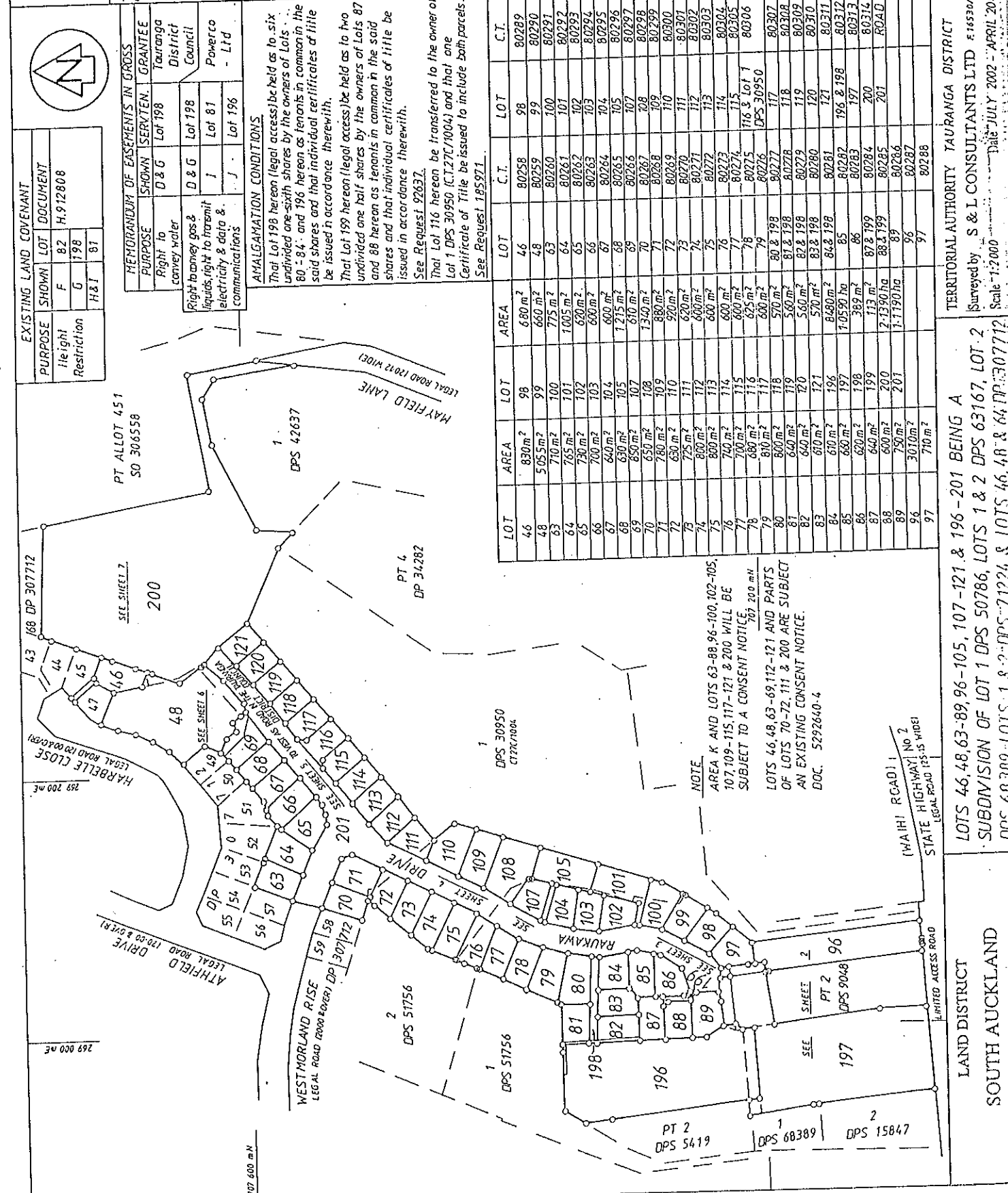
Field Book: \_\_\_\_\_ Traverse Book: \_\_\_\_\_

Reference Plan: \_\_\_\_\_

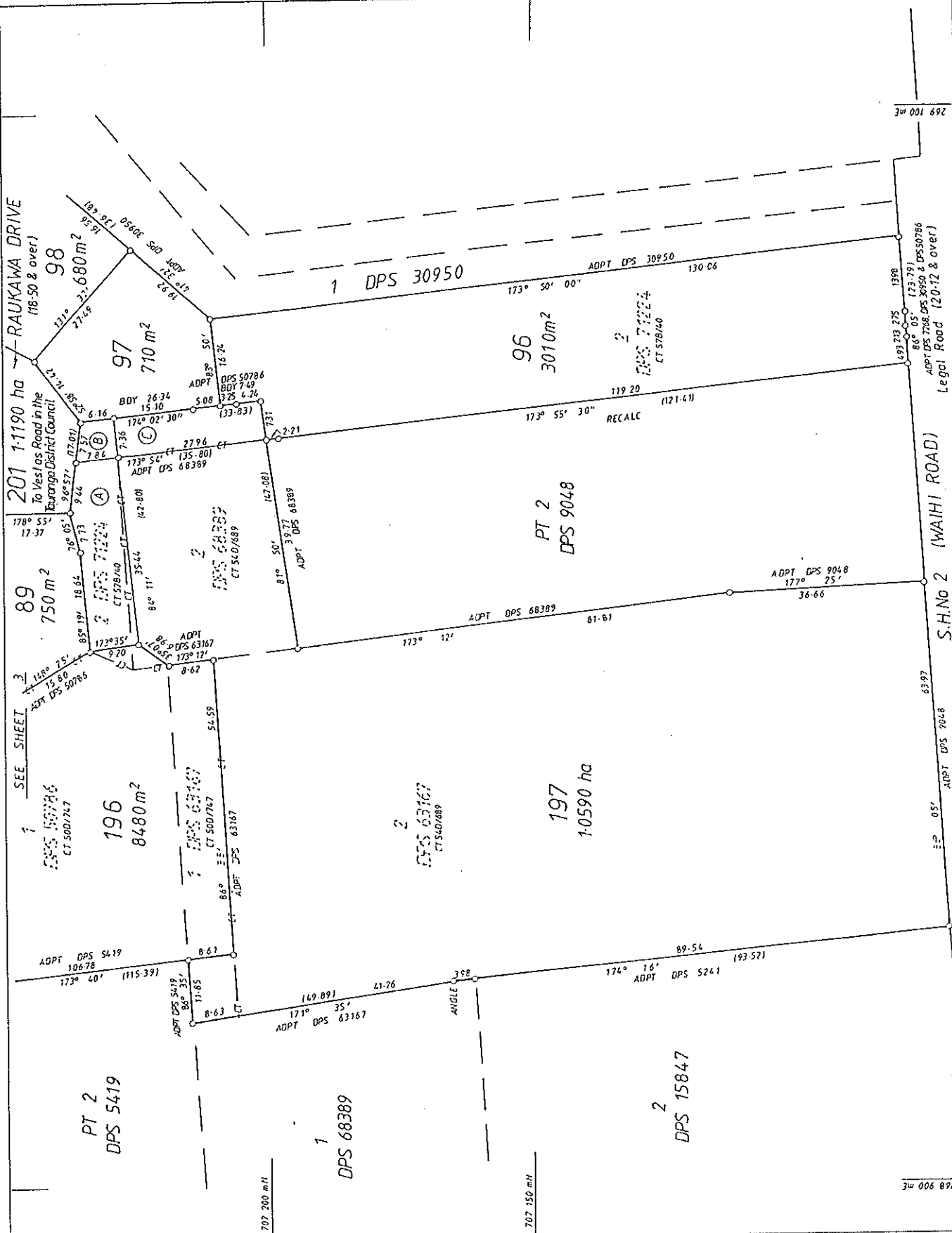
Enacted: \_\_\_\_\_

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand



Approvals



Class of Survey: I

Total Area

Comprised in

JOHN DAVID BARNES

being a person entitled to practice as a licensed cadastral surveyor certify that  
(a) The surveys to which this dataset relates are accurate, and were undertaken by me or under my direction in accordance with the  
Cadastral Survey Act 2002 and the Surveyor General's Rules for  
Cadastral Survey 2002/2;  
(b) This dataset is accurate and has been created in accordance with  
that Act and those Rules.

Signed

Field Book

Reference Plan

Examined

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand

File

Accepted  
Date

TERRITORIAL AUTHORITY TAURANGA DISTRICT  
Surveyed by S & L CONSULTANTS LTD E16530/2  
Date JULY 02 - APRIL 03  
Scale 1:500

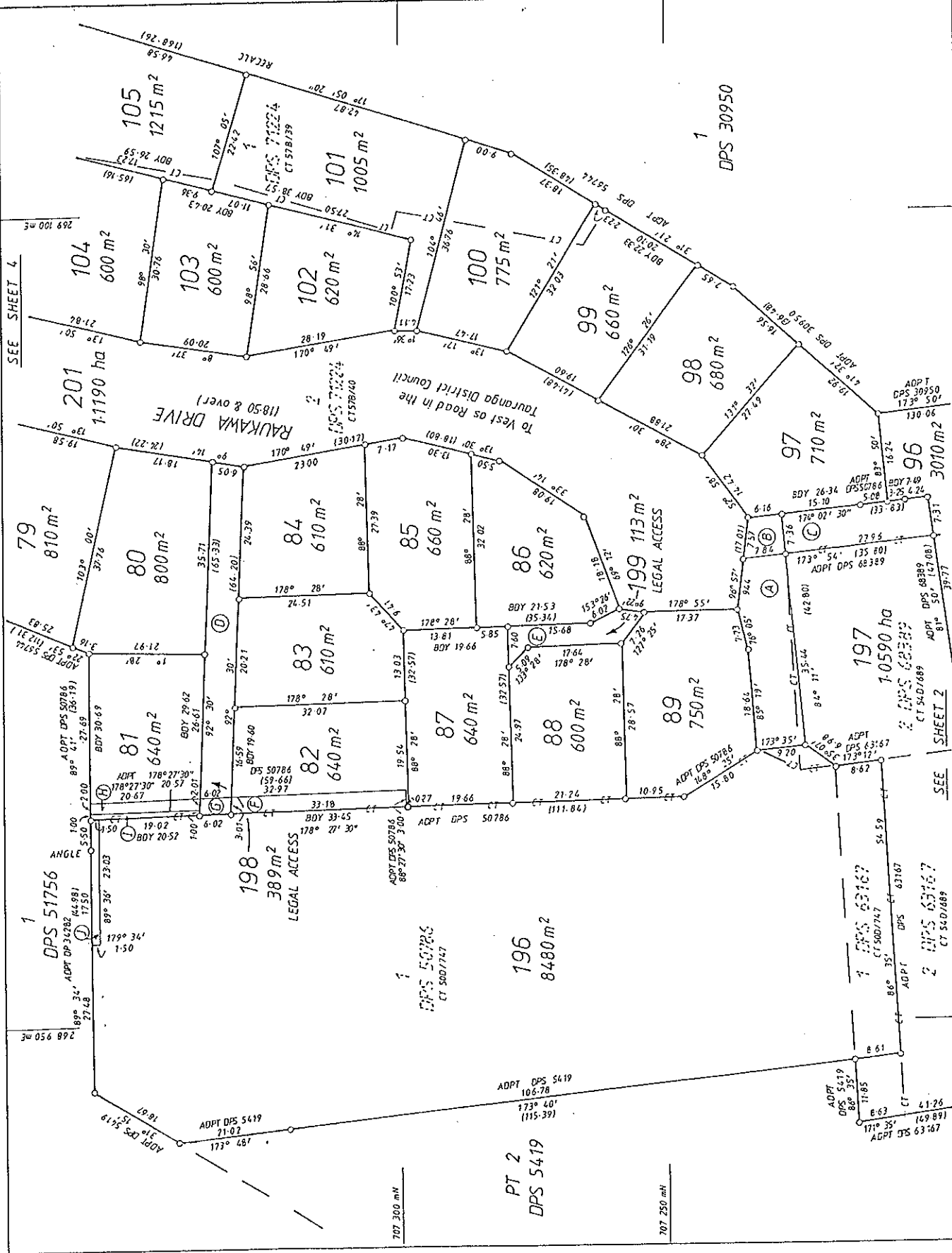
LOTS 46, 48, 63-89, 96-105, 107-121 & 196-201 BEING A  
SUBDIVISION OF LOT 1 DPS 50786, LOTS 1 & 2 DPS 63167, LOT 2  
DPS 68389, LOTS 1 & 2 DPS 71224 & LOTS 46, 48 & 64 DP 307712.

LAND DISTRICT  
SOUTH AUCKLAND

Sheet 2 of 7 DP 320267

Approvals

SEE SHEET 4



Class of Survey: I

Total Area

Comprised in

JOHN DAVID BARNES

being a person entitled to practice as a licensed cadastral surveyor under the provisions of the Survey Act 2002 and the Survey General's Rules for Cadastral Survey 2002/22;  
(b) This document is accurate and has been created in accordance with the Act and those Rules.

Field Book

Reference Plans

Examined

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand

File

Received

Reference

TERRITORIAL AUTHORITY TAURANGA DISTRICT

Surveyed by S & L CONSULTANTS LTD F16530/2

Date JULY 02 - APRIL 03

Scale 1:500

LOTS 46, 48, 63-89, 96-105, 107-121 & 196-201 BEING A SUBDN

OF LOT 1 DPS 50786, LOTS 1 & 2 DPS 63167, LOT 2 DPS 68389,

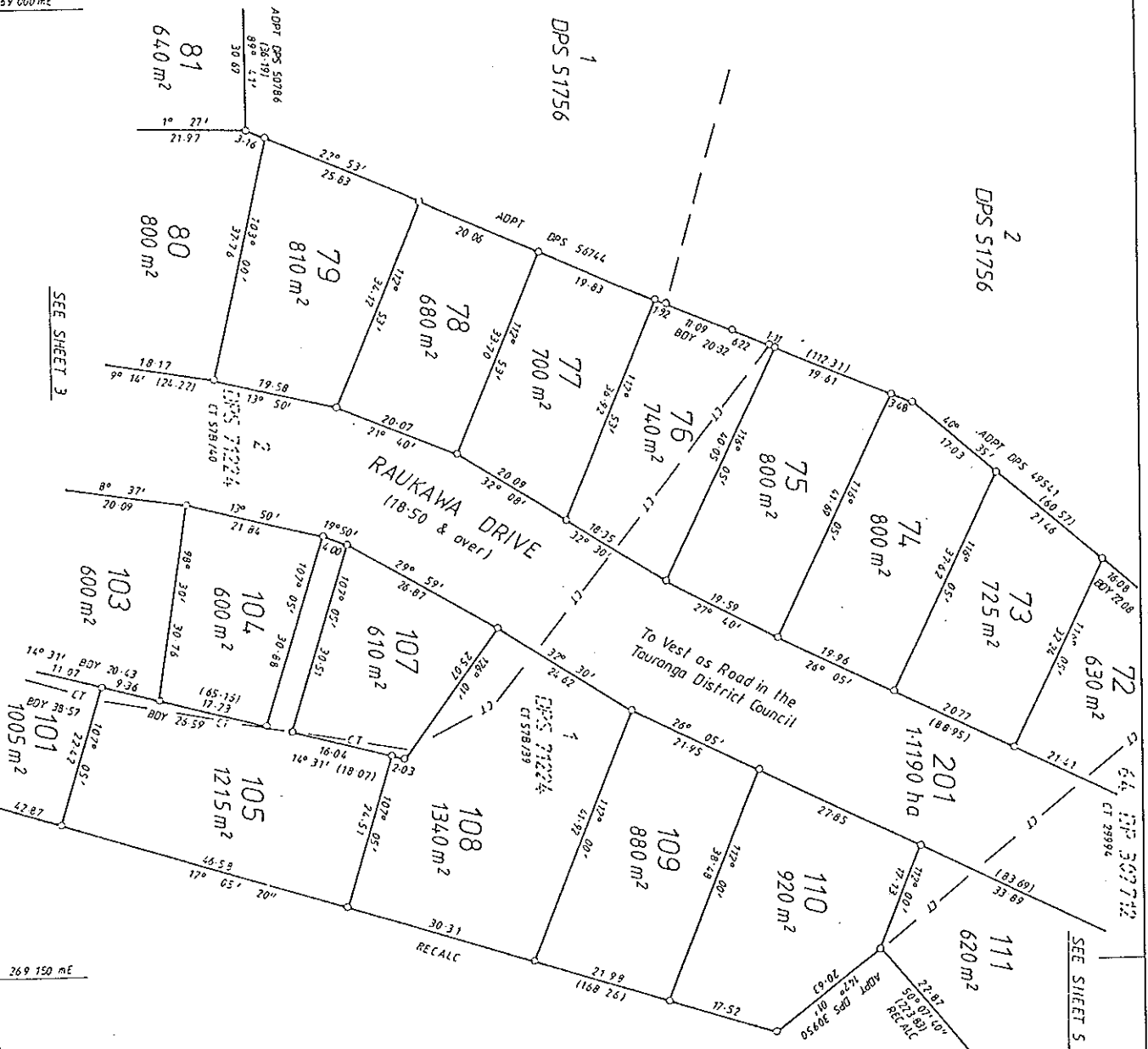
LOTS 1 & 2 DPS 71224 & LOTS 46, 48 & 64 DP 307712.

LAND DISTRICT

SOUTH AUCKLAND

Sheet 1 of 1 NIP 990917





Approvals

Class of Survey: 1

Total Area

Comprised in

JOHN DAVID BARNES

(a) The surveyor is to provide as a licensed cadastral surveyor certify that the survey is to be made in accordance with the provisions of the Survey Act 1980 and the Survey Regulations 1980. (b) This instrument is accurate and has been created in accordance with the Act and those Rules.

Field Book

Reference Plan

Examined

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand

LAND DISTRICT SOUTH AUCKLAND  
LOTS 46, 48, 63-89, 96-105, 107-121 & 196-201 BEING A SUBDN OF LOT 1 DPS 50786, LOTS 1 & 2 DPS 63167, LOT 2 DPS 68389, LOTS 1 & 2 DPS 71224 & LOTS 46, 48 & 64, DP 307712.

TERITORIAL AUTHORITY TAURANGA DISTRICT  
Surveyed by S & L CONSULTANTS LTD F16530/2  
Scale 1:500  
Date JULY 02 - APRIL 03

Approvals

Class of Survey: I

Total Area

Comprised in

JOHN DAVID BARNES

being a person entitled to practice as a licensed cadastral surveyor certify that:  
(a) The surveys to which this plan relates are accurate, and were undertaken by me or under my direction in accordance with the Cadastral Survey Act 2002 and the Surveyor General's Rules for Cadastral Survey 2002;  
(b) This dataset is accurate and has been created in accordance with the Act and these Rules.

Signed

Date

Field Book

Reference Plan

Examined

Corrections

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand

File Number

Accession

TERRITORIAL AUTHORITY TAURANGA DISTRICT

Surveyed by S & L CONSULTANTS LTD F16530/2

Date JULY 02 - APRIL 03

Scale 1:500

LOTS 46, 48, 63 - 89, 96 - 105, 107 - 121 & 196 - 201 BEING A SUBDN

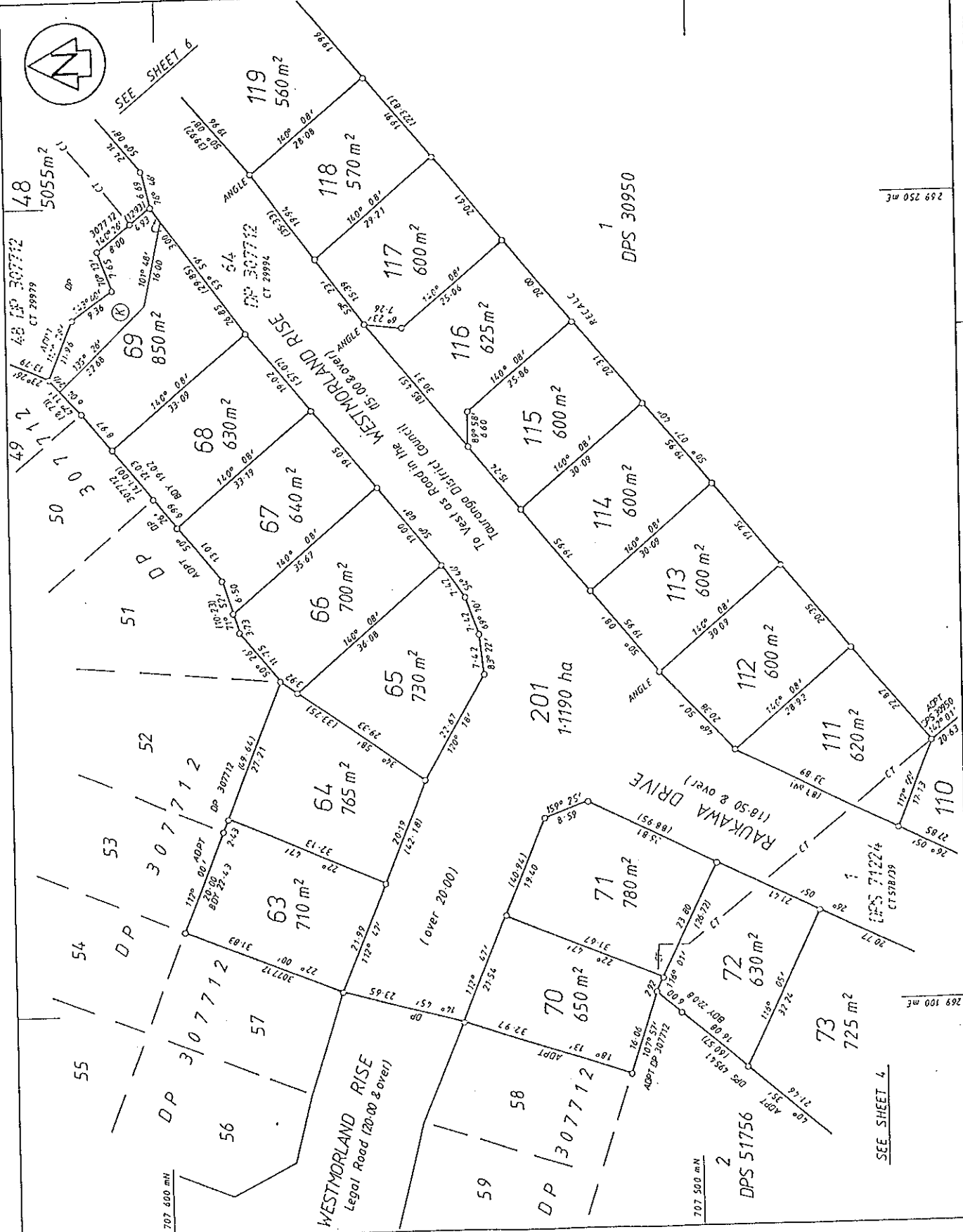
OF LOT 1 DPS 50786, LOTS 1 & 2 DPS 63167, LOT 2 DPS 68389,

LOTS 1 & 2 DPS 71224 & LOTS 46, 48 & 64 DP 307712.

LAND DISTRICT

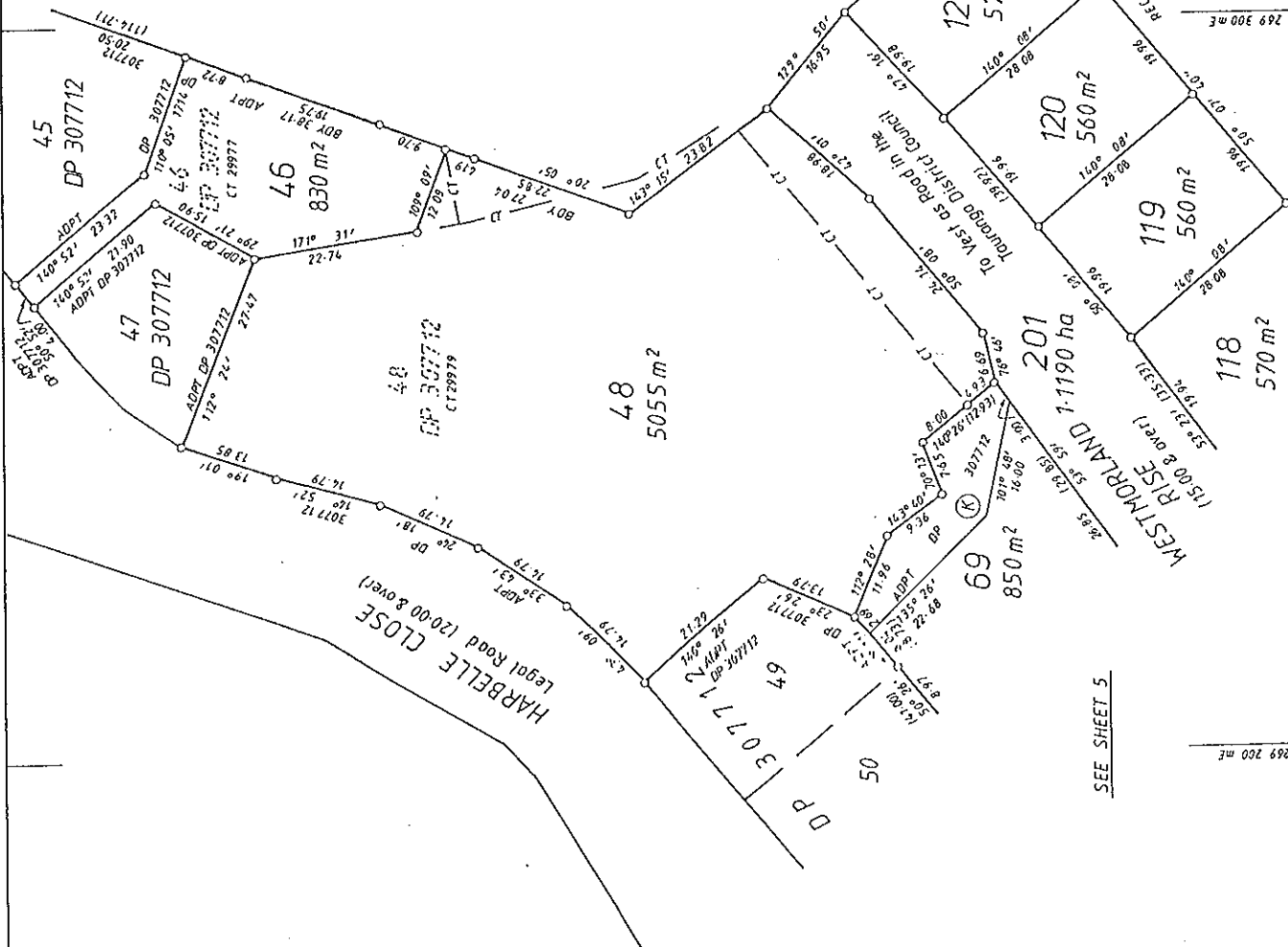
SOUTH AUCKLAND

Sheet 5 of 7 DP 370712



Approvals

SEE SHEET 7



SEE SHEET 7

SEE SHEET 5

Class of Survey: I

Total Area

Comprised in

JOHN DAVID BARNES

being a person entitled to practice as a licensed cadastral surveyor (entirely) that  
(a) The surveys to which this document relates are accurate, and were  
undertaken by me or under my direction in accordance with the  
Cadastral Survey Act 2002 and the Surveyor General's Rules for  
Cadastral Survey 2002;  
(b) This document is accurate and has been created in accordance with  
the Act and those Rules.

Signed

Field Book

Reference Plans

Excluded

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand

PA

Received

Issued

TERRITORIAL AUTHORITY TAURANGA DISTRICT

Surveyed by S & L CONSULTANTS LTD F16530/12

Date JULY 02 - APRIL 03

Scale 1:500

LOTS 46, 48, 63-89, 96-105, 107-121 & 196-201 BEING A SUBDIVISION OF LOT 1 DPS 50786, LOTS 1 & 2 DPS 63167, LOT 2 DPS 68389, LOTS 1 & 2 DPS 71224 & LOTS 46, 48 & 64 DP 307712.

LAND DISTRICT SOUTH AUCKLAND

Sheet 6 of 7 DP 307712

Approvals

PT ALLOT 451  
SO 306558

68  
DP 307712

64  
DP 307712  
CT 29994

200  
2.1390 ha

48  
5055m<sup>2</sup>

48  
DP 307712  
CT 29979

1  
DPS 42637

1  
DPS30950

PT 4  
DP 34282

MAYFIELD  
LANE  
Legal Road  
(20.12 wide)

LOTS 46, 48, 63-89, 96-105, 107-121 & 196-201 BEING A  
SUBDIVISION OF LOT 1 DPS 50786, LOTS 1 & 2 DPS 63167, LOT 2  
DPS 68389, LOTS 1 & 2 DPS 71224 & LOTS 46, 48 & 64 DP 307712.

LAND DISTRICT  
SOUTH AUCKLAND

TERRITORIAL AUTHORITY TAURANGA DISTRICT  
Surveyed by S & L CONSULTANTS LTD E1633012  
Date JULY 02 - APRIL 03  
Scale 1:750

Class of Survey: I

Total Area

Comprised in

JOHN DAVID BARNES

being a person entitled to practise as a licensed cadastral surveyor under the  
(a) The surveys to which this dataset relates are accurate, and were  
undertaken by me or under my direction in accordance with the  
Cadastral Survey Act 2002 and the Surveyor General's Rules for  
Cadastral Survey 2002;  
(b) This dataset is accurate and has been created in accordance with  
that Act and those Rules.

Signed

Field Book

Reference Plan

Examined

Approval as to Survey by Land Information New Zealand

Deposited by Land Information New Zealand

File

Received

Issued

Sheet 7 of 7 DP 320267

WALL HIGHERS AND  
EVIDENCE DEPT.  
CHECKED BY THE WA  
DEPARTMENT, THE STATE  
ADMINISTRATIVE  
DEPARTMENT.

Inspected By	Inspected No.	DESCRIPTION	NAME	DATE
Reviewed				
Designed			Ward	7/03
Drawn			Ward	7/03
Checked				
Approved				



**S & L CONSULTANTS:**  
SURVEYORS - ENGINEERS - PLANNERS

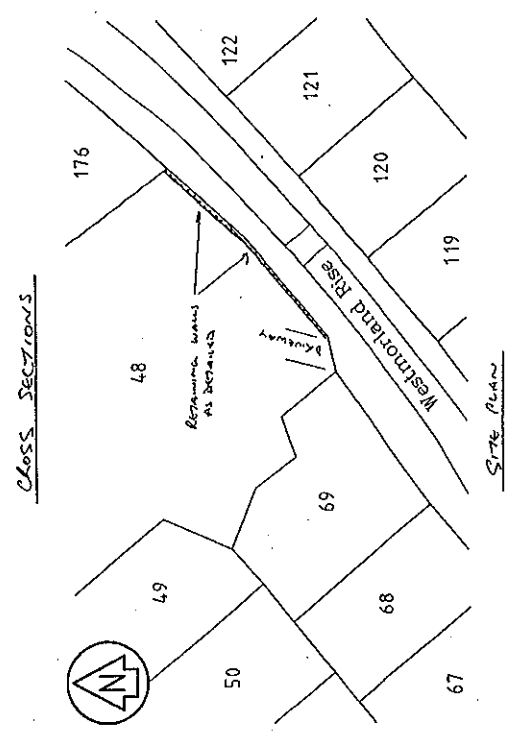
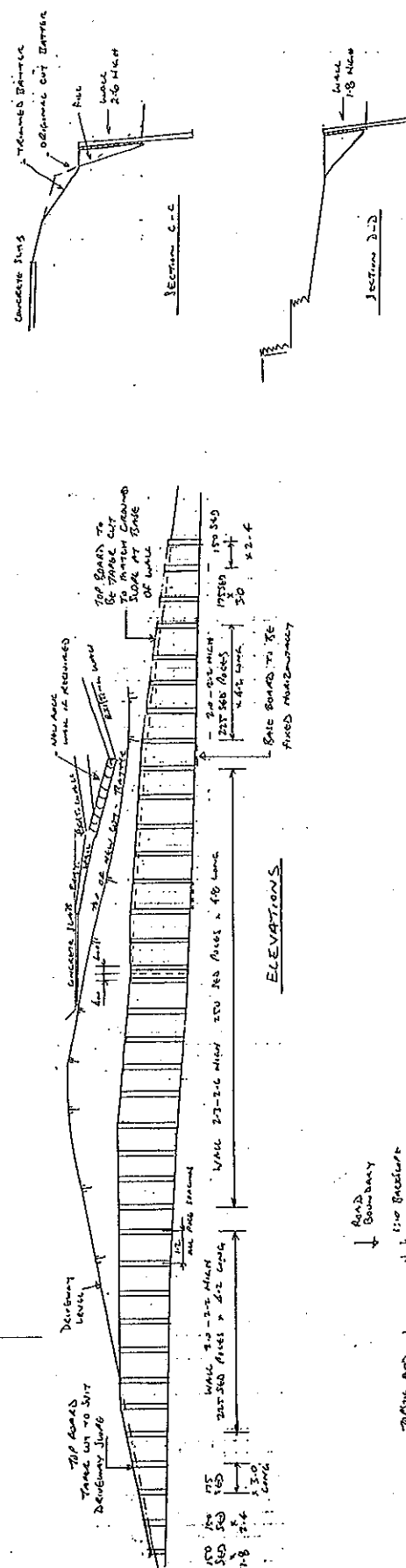
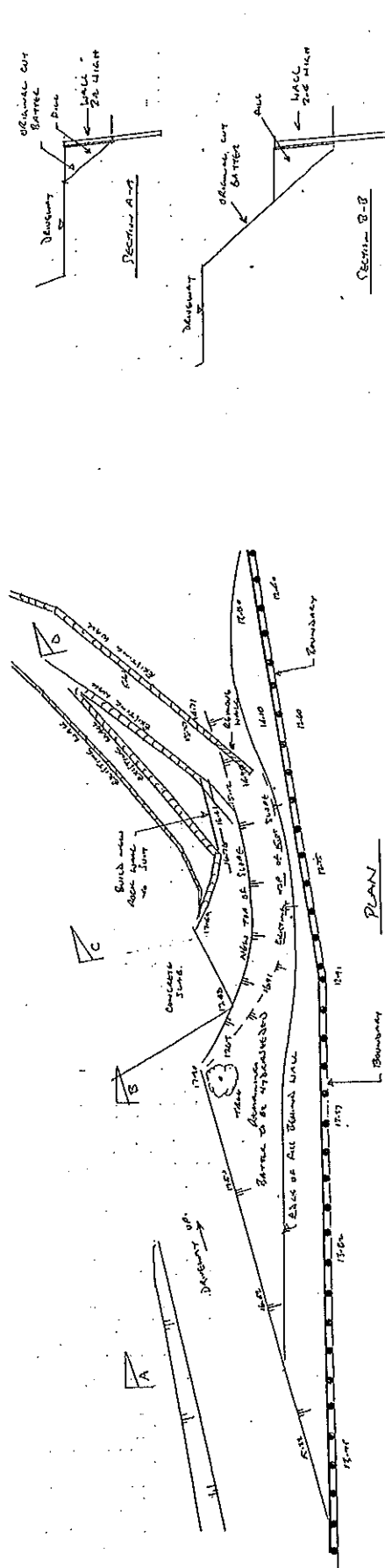
111 Cameron Road, Taunton, Nev  
P.O. Box 231 Ph (07) 077-60  
Fax (07) 077-6005  
Email: [info@taunton.com](mailto:info@taunton.com)

TABLE 1

MAYFIELD SUBDIV  
PAGE 2

ROADSIDE RETAINING  
WALL DETAILS  
LOT 48.

	DRAWING NO	1630-41
	Scale:	0
5	(41)	Copyright © 1987 by The McGraw-Hill Companies, Inc.



1:10 BROOKLET  
 TOP BOUND TO BE  
 TRIPLE CUT WHITE  
 TO ARRIVE BORE AT BASE  
 AT 1/2 OFC.  
 7mm 15 Fcct  
 BOTTOM BOUNDS TO  
 BE Fcct INTERGRALLY  
 LEVEL TO THE TELLON  
 GROUND LEVEL AT BASE  
 OF WALL  
 500 DIA  
 ADJUSTED FACE  
 5" DIA CONCRETE  
 INFILL

FACE LENGTH IN	FACE LENGTH IN	NO ADJUSTED
1.8	1	
2.6	3	
2.0	3	
1.2	13	
2.8	17	

1:10 BROOKLET  
 TOP BOUND TO BE  
 TRIPLE CUT WHITE  
 TO ARRIVE BORE AT BASE  
 AT 1/2 OFC.  
 7mm 15 Fcct  
 BOTTOM BOUNDS TO  
 BE Fcct INTERGRALLY  
 LEVEL TO THE TELLON  
 GROUND LEVEL AT BASE  
 OF WALL  
 500 DIA  
 ADJUSTED FACE  
 5" DIA CONCRETE  
 INFILL

FACE LENGTH IN	FACE LENGTH IN	NO ADJUSTED
1.8	1	
2.6	3	
2.0	3	
1.2	13	
2.8	17	

### Typical Details

NOTE

WALL HEIGHTS AND  
FACED HEIGHTS  
CHECKED BY THE  
DESIGNER ON SITE  
FORWARD AND  
REVERSE

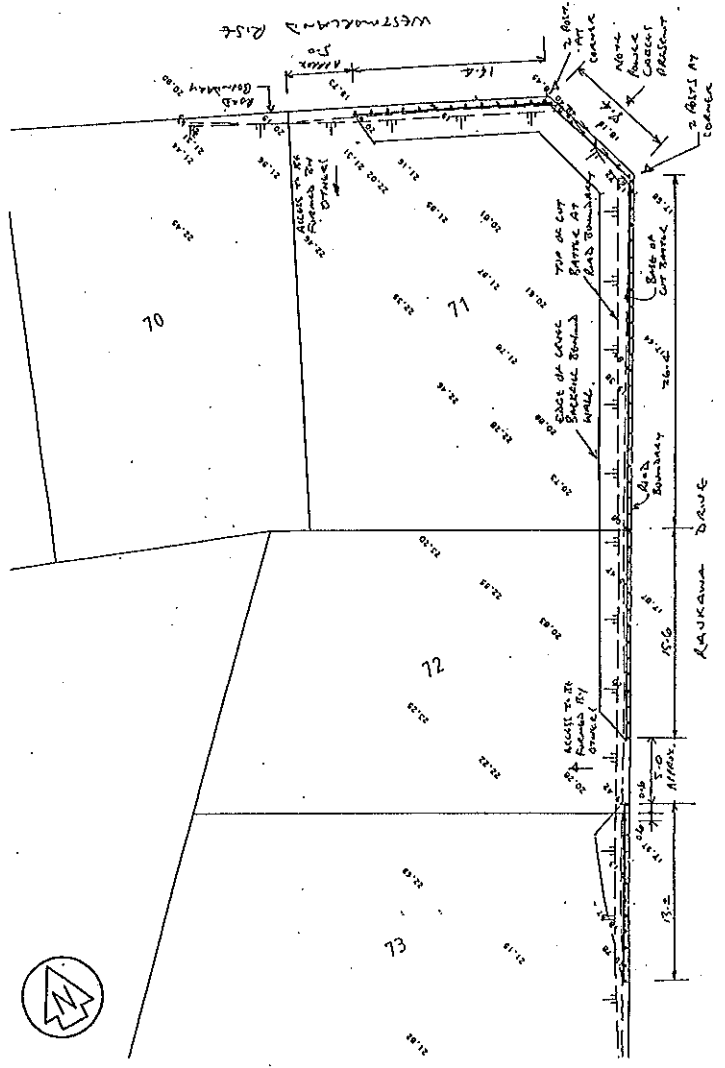
NO.	DATE	REVISION
1	10/10/10	ISSUED FOR TENDERS
2	11/10/10	REVISED TO SHOW WALL HEIGHTS
3	12/10/10	REVISED TO SHOW POLE HEIGHTS
4	13/10/10	REVISED TO SHOW POLE DIAMETERS
5	14/10/10	REVISED TO SHOW POLE SPACING
6	15/10/10	REVISED TO SHOW POLE WEIGHT
7	16/10/10	REVISED TO SHOW POLE STRENGTH
8	17/10/10	REVISED TO SHOW POLE TOLERANCE
9	18/10/10	REVISED TO SHOW POLE DEFLECTION
10	19/10/10	REVISED TO SHOW POLE VIBRATION



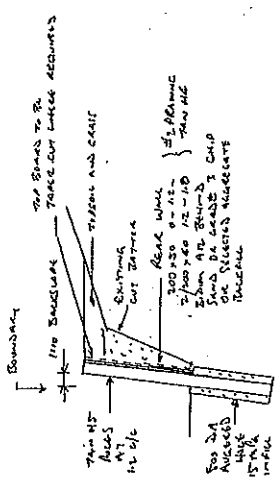
S&L CONSULTANT  
SURVEYORS - ENGINEERS - PL  
111 Commercial Road, Teluk Anson, N  
P.O. Box 231, Teluk Anson, N  
Tel: 07777-0000  
Fax: 07777-0000  
Email: s&l@com.net

TITLE  
MAYFIELD SUBD.  
STAGE 2  
ROADSIDE RETA  
WALL DETAILS  
LOTS 71, 72, 73

Copyright in this drawing is reserved  
ORIGINAL SCALE  
1:500, 1:100, 1:200  
DRAWING NO  
16530-42  
Revision 10

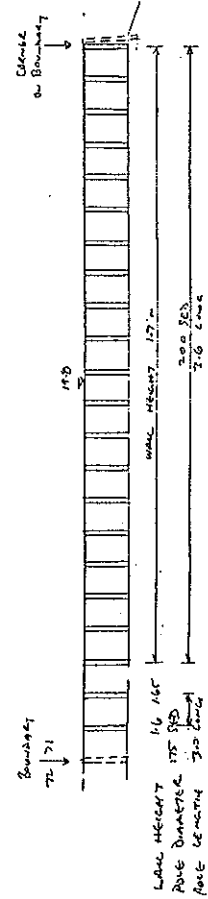
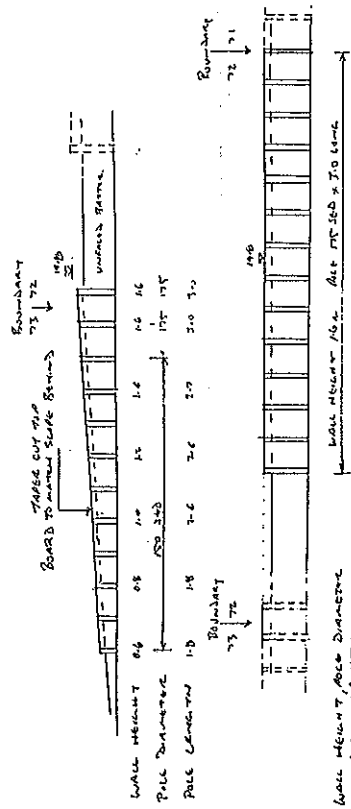


SITE PLAN



WALL HEIGHT	POLE LENGTH	NO. OF POLES
0.6 TO 0.8	1.5	6
1.0 TO 1.2	2.0	8
1.4 TO 1.6	2.5	10
1.8 TO 2.0	3.0	12
2.2 TO 2.4	3.5	14
2.6 TO 2.8	4.0	16
3.0 TO 3.2	4.5	18
3.4 TO 3.6	5.0	20

TYPICAL DETAILS



ELEVATIONS

GRAPHIC SCALE

## **APPENDIX II**

**Statement of Professional Opinion as to the Suitability of  
Land for Building Development**

**Lot Summary Report**

## SECTION 2

To: The Director of Environmental Services

### STATEMENT OF PROFESSIONAL OPINION AS TO THE GEOTECHNICAL SUITABILITY OF LAND FOR BUILDING

DEVELOPMENT: Mayfield Subdivision Stage 2

OWNER: Mayfield Ltd

LOCATION: Westmorland Rise, Raukawa Drive, Bethlehem

I Michael William Hughes of S & L Consultants Ltd

(Full Name)

PO Box 231, Tauranga

(Name and Address of Firm)

Hereby confirm that;

- 1) I am a professional person appropriately qualified with experience in geotechnical engineering to ascertain the suitability of the land for building development and was retained as the Soils Engineer to the above development.
- 2) An appropriate level of site investigation and construction supervision has been carried out under my direction and is described in my development evaluation dated 5 May 2003
- 3) In my professional opinion, not to be construed as a guarantee, I consider that;
  - (a) The area shown in my report dated 5 May 2003 of each new allotment is suitable for the erection thereon of the building types appropriate to the zoning of the land, provided that;  
Recommendations contained in my report are complied with.
  - (b) The structural earth fills shown on the attached Plan Nos. 16530-51 and 52 have been placed in accordance with the Code of Practice for Development of the Tauranga District Council.
  - (c) The completed works give due regard to all land slope and foundation stability considerations.
  - (d) The filled ground is suitable for the erection thereon of residential buildings not requiring specific design in terms of NZS 3604:1999 and related documents providing that;  
Recommendations contained in my report, section 6 are complied with.
  - (e) The original ground not affected by filling is suitable for the erection thereon of residential buildings not requiring specific design in terms of NZS 3604:1999 and related documents, subject to the recommendations contained in my report including those relating to topsoil depths and soil variations away from test or observation positions.
4. This professional opinion is furnished to the Council and the owner for their purpose alone, on the express condition that it will not be relied upon by any other person and does not remove the necessity for the normal inspection of foundation conditions at the time of erection for any dwelling.

Signed 

Date 5 May 2003



TAURANGA  
DISTRICT COUNCIL

**SUITABILITY OF LAND  
FOR BUILDING DEVELOPMENT**

**TAURANGA DISTRICT COUNCIL**

MAY 98

G2 Δ





## LOT SUMMARY REPORT

TAURANGA DISTRICT COUNCIL

MAY 98

G 2a Δ

### MAYFIELD SUBDIVISION STAGE 2 WESTMORELAND RISE, RAUKAWA DRIVE, BETHLEHEM

The comments and notations included on this summary sheet are outlined in the support documents.  
These shall be read in conjunction with this summary.

TDC Sub 5311

File Ref: 16530

Lot#	Area(m <sup>2</sup> )	Subsurface Data						Foundations		Building line restriction?	Recommendations/restrictions
		Shear Strength kPa	Subdivision Filling		Natural topography unworked	Natural topography earthworked		Conventional shallow Foundations to NZS 3604:1999	Specific Design		
			Y/N	Depth (m)		Y/N	Depth(m)				
46	830	-	-	-	-	-	-	Y	N	N	Refer to Connell Wagner report for Stage 1 - minor boundary adjustment only undertaken in Stage 2.
48	5055	-	-	-	-	-	-	Y	N	N	
63	710	130	N		Y	N		Y	N	N	
64	765	175	N		N	Y	0.5	Y	N	N	
65	730	175	N		N	Y	2.0	Y	N	N	
66	700	170	N		N	Y	2.0	Y	N	N	
67	640	175	N		N	Y	2.0	Y	N	N	
68	630	175	N		N	Y	2.5	Y	N	N	
69	850	175	N		N	Y	2.5	Y	N	Y	
70	650	175	N		N	Y	0.5	Y	N	N	
71	780	175	N		N	Y	2.0	Y	N	N	
72	630	175	N		N	Y	1.0	Y	N	N	
73	725	142	Y	0.5	N	Y	1.0	Y	N	N	
74	800	160	Y	1.0	N	Y	0.5	Y	N	N	
75	800	175	N		N	Y	0.3	Y	N	N	
76	740	148	N		N	Y	0.5	Y	N	N	
77	700	175	N		N	Y	0.5	Y	N	N	
78	680	142	N		N	Y	0.5	Y	N	N	
79	810	148	N		N	Y	0.5	Y	N	N	
80	800	143	Y	0.5	N	Y	0.5	Y	N	N	
81	640	115	N		Y	N		Y	N	N	
82	640	135	N		N	Y	0.3	Y	N	N	
83	610	89	Y	0.5	N	Y	0.3	Y	N	N	
84	610	175	Y	1.0	N	Y	0.3	Y	N	N	

Comments

Refer to S & L Consultants Ltd report reference 16530 dated 5 May 2003. Lots shown on DP 320267.

\* Taken from post subdivision boreholes at likely foundation depth of 400mm from existing topsoiled ground levels.

**MAYFIELD SUBDIVISION STAGE 2  
WESTMORELAND RISE, RAUKAWA DRIVE, BETHLEHEM**

The comments and notations included on this summary sheet are outlined in the support documents. These shall be read in conjunction with this summary.

**TDC Sub** 5311

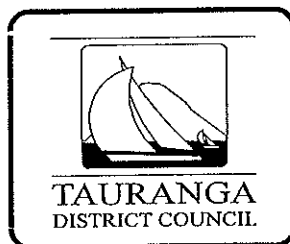
**File Ref:** 16530

Lot#	Area(m <sup>2</sup> )	Subsurface Data						Foundations		Building line restriction?	Recommendations/restrictions/comments
		Shear Strength kPa	Subdivision Filling		Natural topography unworked	Natural topography earthworked		Conventional shallow Foundations to NZS 3604:1999	Specific Design		
			Y/N	Depth (m)		Y/N	Depth(m)				
85	660	175	Y	1.0	N	Y	0.5	Y	N	N	
86	620	175	Y	1.0	Y	N		Y	N	N	
87	640	140	N		N	Y	0.3	Y	N	N	
88	600	111	N		Y	N		Y	N	N	
89	700	-	-		-	-					House on property
96	3010	-	-		-	-					To be further developed
97	710	129	N		Y	N		Y	N	N	
98	680	108	N		Y	N		Y	N	N	
99	660	85	N		N	Y	0.5	Y	N	N	
100	775	150	N		N	Y	0.5	Y	N	N	
101	1005	-	-		-	-					House on property
102	620	145	N		N	Y	0.5	Y	N	N	
103	600	175	N		N	Y	0.5	Y	N	N	
104	600	150	N		N	Y	1.0	Y	N	N	
105	1215	66	N		N	Y	1.0	Y	N	N	
107	610	100	N		N	Y	2.0	Y	N	N	
108	1340	-									House on property
109	880	120	N		Y	N		Y	N	N	Non structural fill present
110	920	74	N		N	Y	0.5	Y	N	N	
111	620	150	Y	0.7	N	Y	0.3	Y	N	N	
112	600	175	Y	0.5	N	Y	0.3	Y	N	N	
113	600	166	Y	1.0	N	Y	0.3	Y	N	N	Filling in temporary silt pond
114	600	150	Y	0.8	N	Y	0.3	Y	N	N	Filling in temporary silt pond
115	600	150	Y	0.8	N	Y	0.3	Y	N	N	Filling in temporary silt pond
116	625	142	Y	0.5	N	Y	0.3	Y	N	N	

Comments

Refer to S & L Consultants Ltd report reference 16530 dated 5 May 2003. Lots shown on DP 320267.

\* Taken from post subdivision boreholes at likely foundation depth of 400mm from existing topsoiled ground levels.



**LOT SUMMARY  
REPORT**

**TAURANGA DISTRICT COUNCIL**

**MAY 98**

**G 2a Δ**

**TAURANGA DISTRICT COUNCIL**

MAY 98

**G 2a  $\Delta$**

TDC Sub 5311

File Ref: 16530

[illegible]

Comments

**C:\Users\jw0167**

\* Taken from post subdivision boreholes at likely foundation depth of 400mm from existing topsoiled ground levels.

## **APPENDIX III**

### **Compaction Test Results**

Our Ref: 16530

## SUMMARY OF COMPACTION TEST RESULTS

Test No	Location	Air Voids Percentage	Undrained Shear Strength kPa
T19	Road	14	UTP
20	Road	11.2	UTP
21	Road	2.7	UTP
22	Road	6.4	>164
23	Road	7.9	UTP
20	Road	5	UTP
25	Road	7.8	>164
26	Lot 118	2.4	>164
27	Lot 119	0.0	>164
28	Lot 120	2.8	>164
29	Lot 120	2.3	UTP
30	Lot 86	5.3	UTP
31	Lot 85	4.9	>164
32	Lot 84	5	UTP
33	Lot 85	5.0	UTP
34	Lot 75	3.1	UTP
35	Lot 74	8.6	UTP
70	Lot 83	11.9	128
71	Lot 83	7.2	UTP

UTP - denotes that the ground was too hard to push in the shear vane

Refer to 16530-51, 52 for test locations

## **APPENDIX IV**

**Post Construction Borehole Logs**

**Pre Construction Borehole & Test Pit Logs**



Borehole No. 63

Sheet: 1 Of: 1

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X	0.0					
Brown CLAYEY SILT slightly sandy moist very stiff almost friable	X	0.2					
	X	0.3					
	X	0.5	135/ 43				
	X	0.8	123/ 32				
	X	1.0					
End of Borehole		1.4	178/ 66				

EXCAVATION METHOD: 5D  $\phi$  Hand Auger

Borehole No. 64

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated:	09-04-03	RL Ground:
-----------------	----------	------------

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X-X	0.0					
Brown CLAYEY SILT slightly sandy moist very stiff	X-X X-X X-X X-X X-X	0.1 0.2 0.3 0.4	172/62				
Brown SILTY med. SAND sl. clayey moist stiff becomes brownish light grey	X-X X-X	0.6 0.7	169/40				
Dark brown CLAYEY SILT moist v. stiff h. plastic grades brown	X-X X-X	0.8 0.9					
End of Borehole		1.0 1.1	152/20				

EXCAVATION METHOD: 50  $\phi$  Hand Auger






Borehole No. 65

Sheet: 1 Of: 1

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level	Undrained Shear Strength (kPa)			
				50	100	150	
Black ORGANIC CLAYEY SILT moist stiff friable	XX	0.0					
Light brownish light grey fine SAND slightly moist medium dense		0.2					
		0.4	UTP				→
		0.7	UTP				→
		1.0					
End of Borehole		1.1	142/38				

EXCAVATION METHOD: 50  $\phi$  Hand Auger

Borehole No. 66

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 09-04-03 RL Ground:

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level	Undrained Shear Strength (kPa)		
				50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X X X	0.0				
Yellowish light brown SANDY SILT sl. clayey moist very stiff	X	0.2				
	X	0.4	16% 18			
	X					
	X					
	X	0.8	12% 24			
Orangeish brown SILT sl. clayey moist very stiff	X	0.9				
End of Borehole		1.0				
		1.1	10% 26			

EXCAVATION METHOD: 5D  $\phi$  Hand Auger



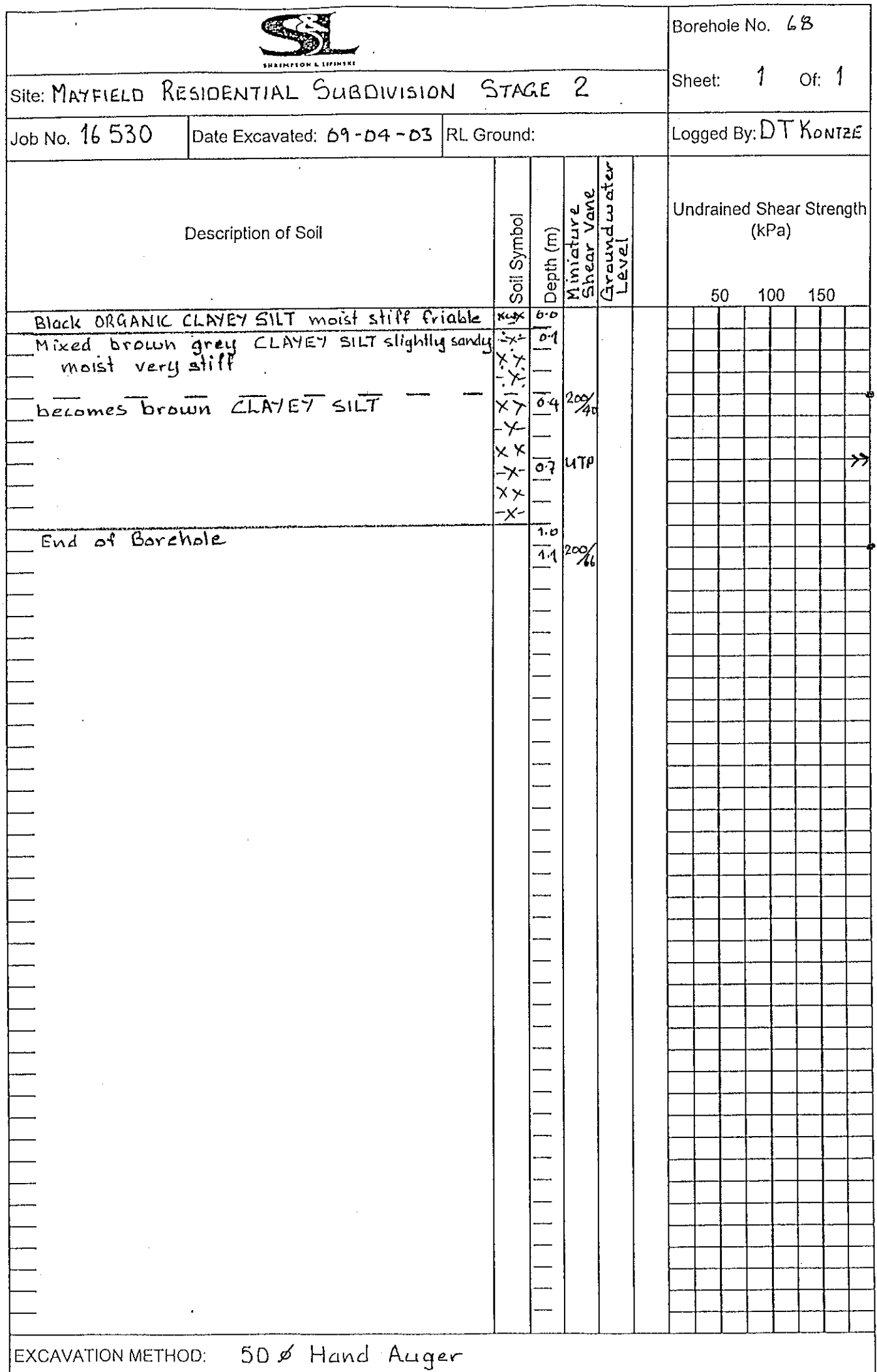
Borehole No. 67

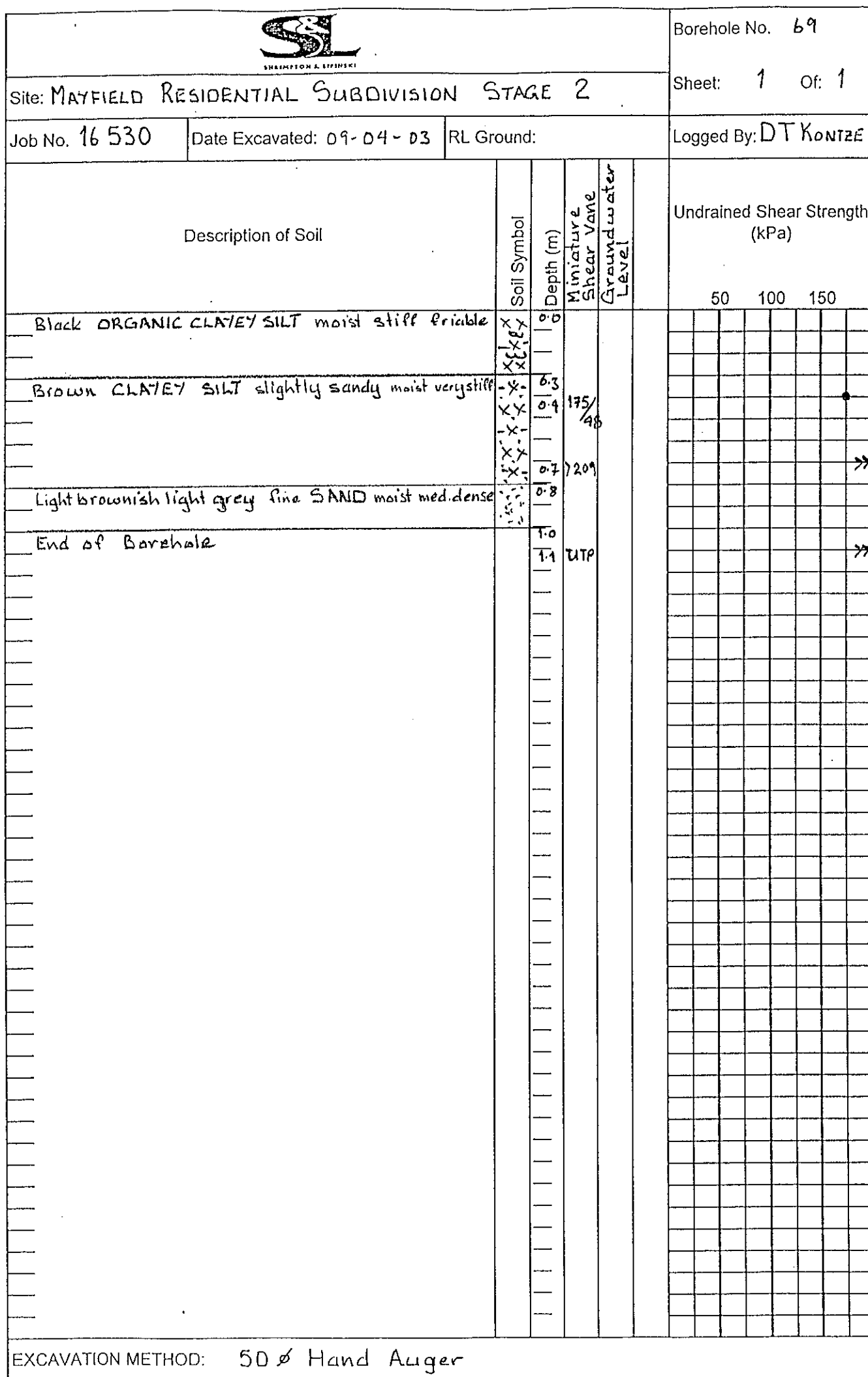
Sheet: 1 Of: 1

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X X -X-	0.0					
Light brown CLAYEY SILT slightly sandy moist very stiff	X X -X-	0.2					
Light brown SILT moist hard	X X X X X X X X X	0.4 0.6	UTP 209				>>
End of Borehole		1.0 1.1	195/48				

EXCAVATION METHOD: 50  $\phi$  Hand Auger





Borehole No. 7D

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 11-b4-b3

RL Ground:

Logged By: DT KONTZE

[illegible]

EXCAVATION METHOD: 50  $\phi$  Hand Auger



Borehole No.	71
--------------	----

Sheet: 1 Of: 1

Date Excavated: 11-04-03  
30-04-03

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Rear							
Black ORGANIC CLAYEY SILT moist stiff friable	X X -X-	0.0					
Brown CLAYEY SILT moist very stiff	X X	0.2					
Light brown SILT moist very stiff	X	0.3					
Light brownish light grey fine SAND moist medium dense	. . . . . .	0.4	>209			→	
Brown CLAYEY SILT moist verystiff plastic	X X -X- X X -X-	0.6 0.7	142/ 35			•	
End of Borehole		1.0 1.1	151/ 32			•	
Front							
Black ORGANIC CLAYEY SILT moist stiff friable	X X -X- X X -X-	0.0 0.2					
layer of vegetation							
Brown SANDY SILT slightly clayey very moist very stiff slightly friable	X X -X- X X -X-	0.4 0.6 0.7	198/ 38			•	
Grades brown CLAYEY SILT slightly sandy very moist verystiff	X X -X-	0.8	128/ 18			•	
End of Borehole		1.0 1.1	91/ 22			•	

EXCAVATION METHOD: 50  $\phi$  Hand Auger

Borehole No. 72

Sheet: 1 Of: 1

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Job No. 16 530

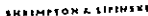
Date Excavated: 11-D 4-D 3

RL Ground:

Logged By: DT KONTZE

[illegible]EXCAVATION METHOD: 50  $\phi$  Hand Auger





Borehole No.	73
--------------	----

Sheet: 1 Of: 1

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Job No. 16 530

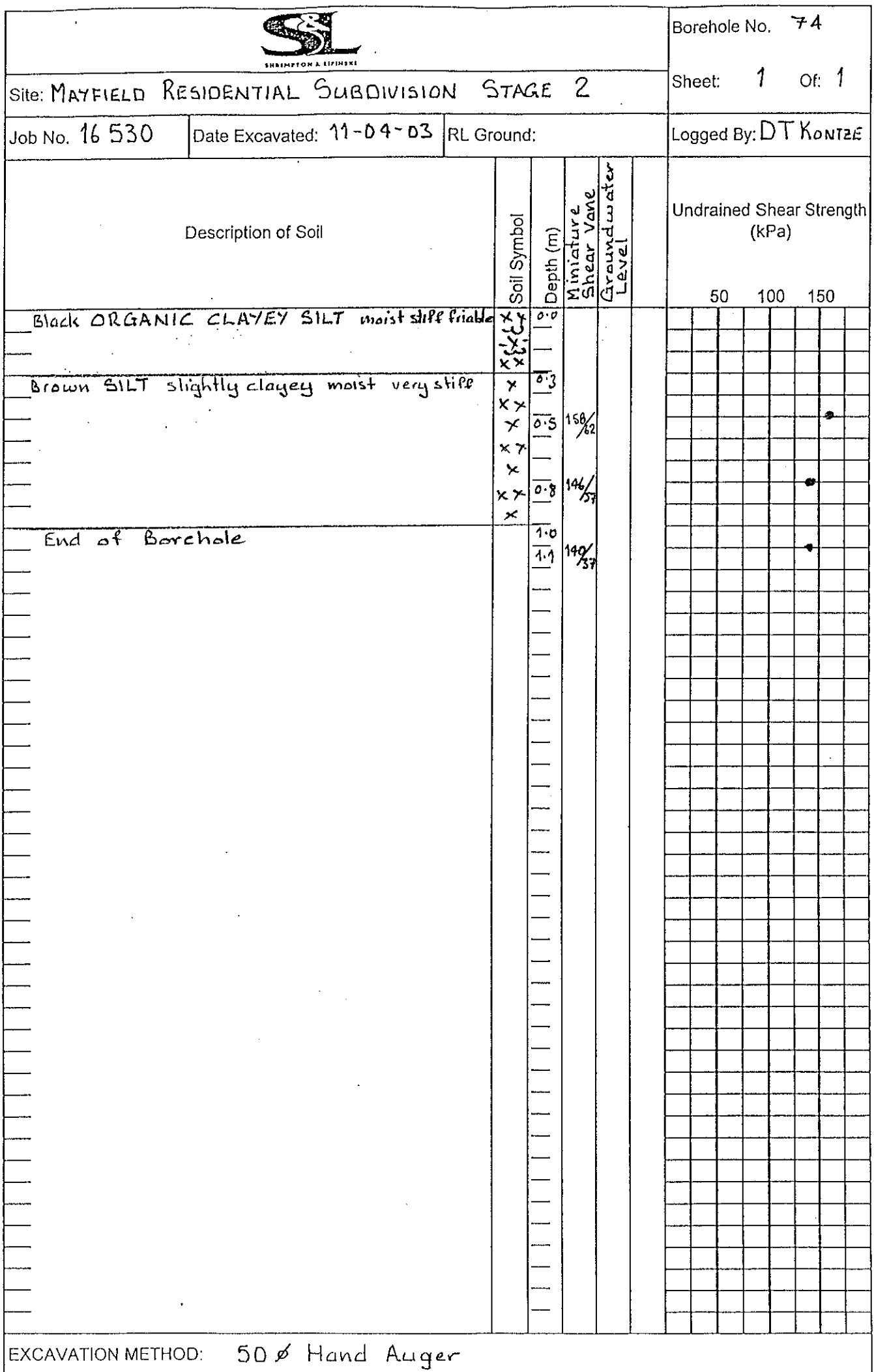
Date Excavated: 11-04-03

RL Ground:

Logged By: DT KONTZE

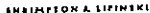
Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level	Undrained Shear Strength (kPa)		
				50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	XX X	0.0				
Brown CLAYEY SILT moist very stiff plastic	XX X	0.2				
	XX X	0.4	142/ 78			
becomes mixed brown	XX X	0.5				
becomes dark brown	XX X	0.6				
Brown SILT moist very stiff	XX X	0.8	154/ 71			
End of Borehole		1.0				
		1.1	148/ 73			

EXCAVATION METHOD: 50  $\phi$  Hand Auger



EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger



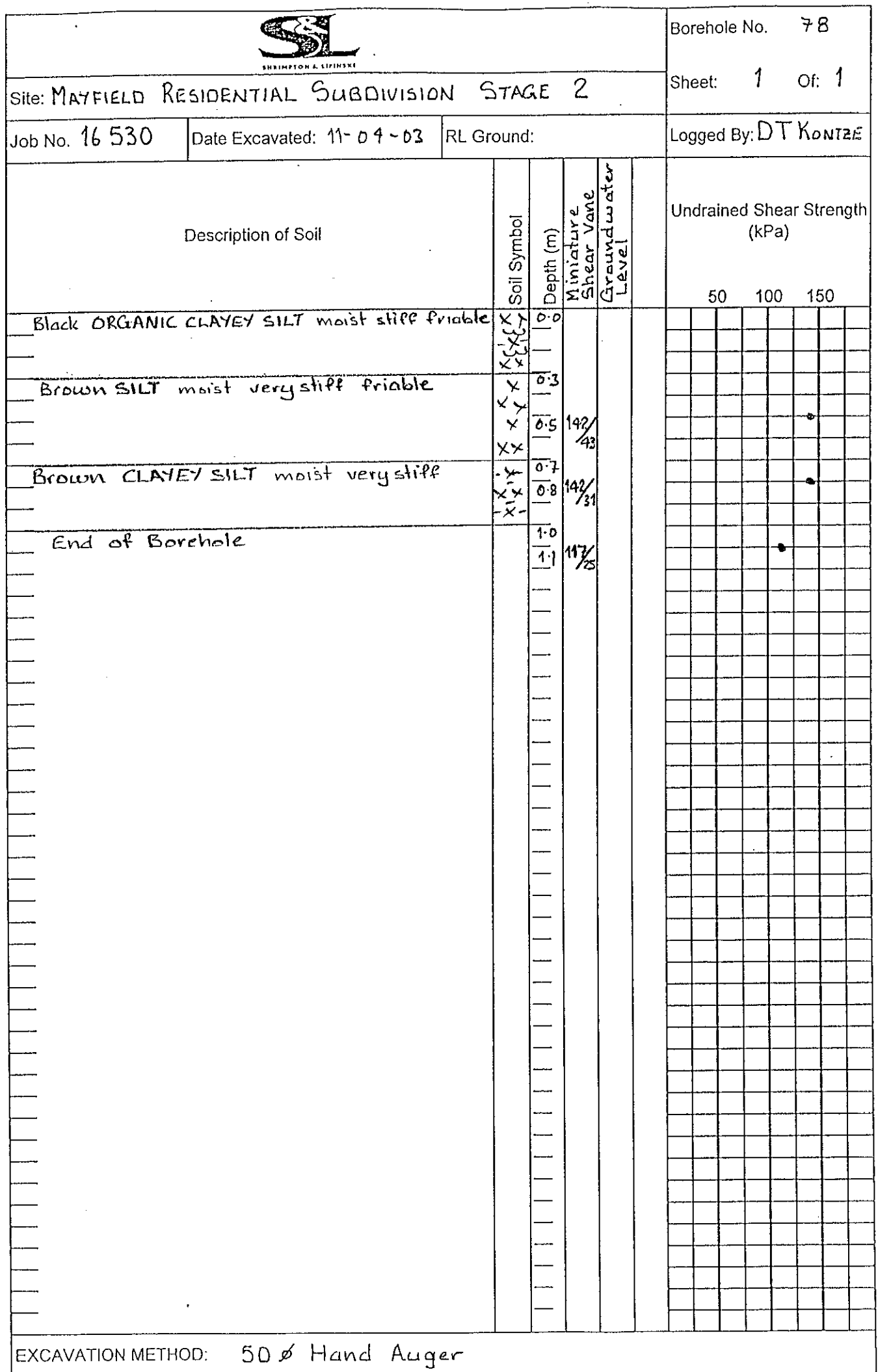
Borehole No. 77

Sheet: 1 Of: 1

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X X - X -	0.0 0.1					
Light brown CLAYEY SILT sl. sandy moist v. stiff	X X X	0.2 0.3					
Brown SILT moist very stiff friable	X X Y X X Y X X X X	0.4 0.5 0.6 0.7 0.8 0.9 1.0	200/ 82				
End of Borehole		1.1	157/ 52				
			146/ 35				

EXCAVATION METHOD: 50  $\phi$  Hand Auger



EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger



Borehole No. 81

Sheet: 1 Of: 1

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Job No. 16 530

Date Excavated: 11-04-03

RL Ground:

Logged By: DT KONIZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level	Undrained Shear Strength (kPa)		
				50	100	150
<b>Upper West</b>						
Black ORGANIC CLAYEY SILT moist stiff friable	X	0.0				
	X	0.1				
	X	0.2				
Brown SILT moist very stiff friable	X	0.3				
	X	0.4				
	X	0.5				
grades slightly clayey	X	0.7				
	X	0.8	130/28			
	X	0.9				
End of Borehole		1.0				
		1.1	135/28			
<b>Lower East</b>						
Black ORGANIC CLAYEY SILT moist stiff friable	X	0.0				
	X	0.1				
	X	0.2				
Brown SILT moist very stiff friable	X	0.3				
	X	0.4				
	X	0.5				
grades slightly clayey	X	0.7	115/25			
	X	0.8				
	X	0.9				
End of Borehole		1.0				
		1.1	151/34			

EXCAVATION METHOD: 50  $\phi$  Hand Auger



Borehole No. 82

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 of 1

Job No. 16 530

Date Excavated: 11-04-03

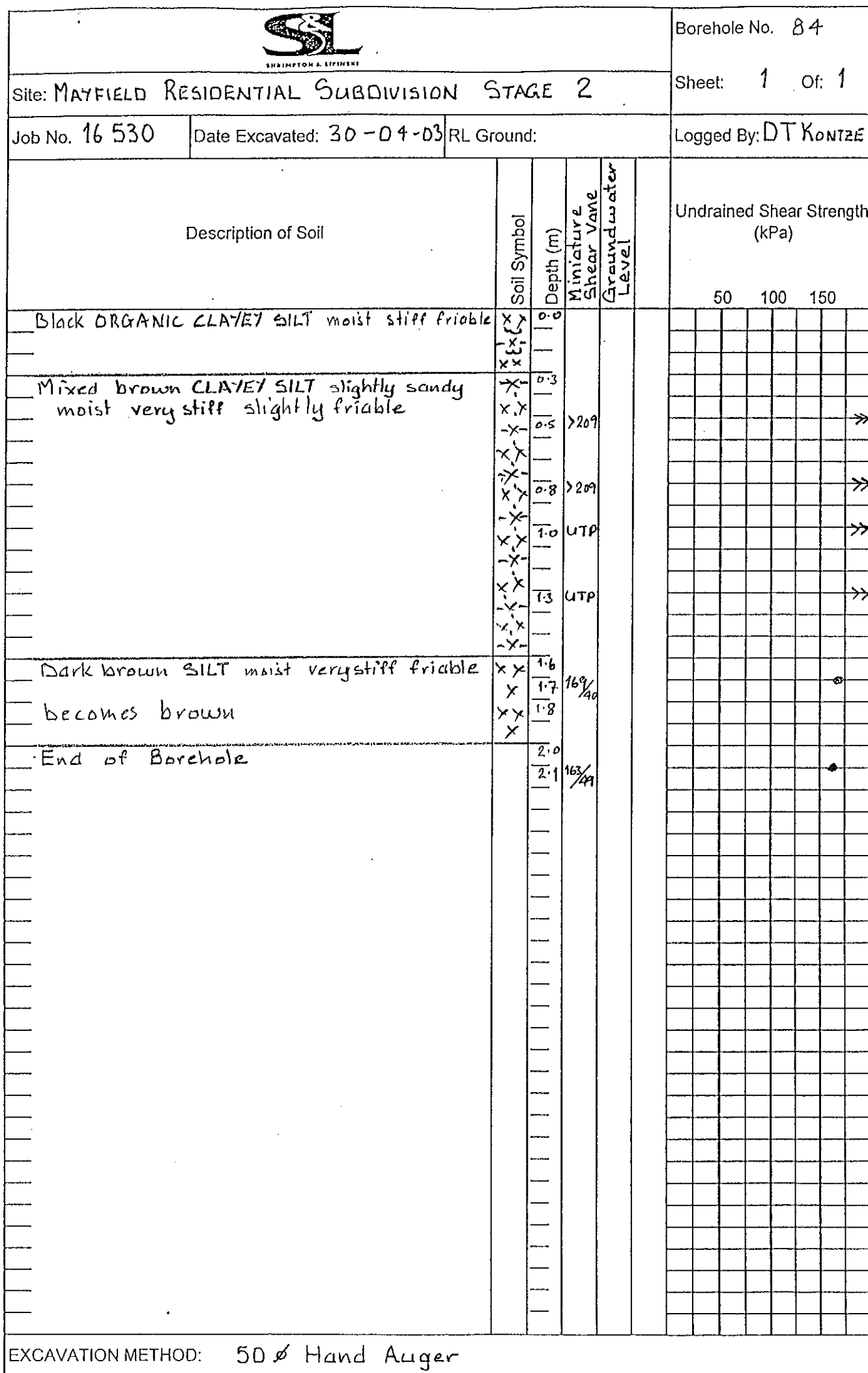
RL Ground:

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X	0.0					
	X	0.1					
	X	0.2					
Brown SILT moist very stiff friable	X	0.3					
	X	0.4	135/ 34				
	X	0.5					
	X	0.6					
grades slightly clayey	X	0.7					
	X	0.8	135/ 28				
	X	0.9					
End of Borehole		1.0					
		1.1	145/ 16				
		1.2					
		1.3					
		1.4					
		1.5					
		1.6					
		1.7					
		1.8					
		1.9					
		2.0					
		2.1					
		2.2					
		2.3					
		2.4					
		2.5					
		2.6					
		2.7					
		2.8					
		2.9					
		3.0					
		3.1					
		3.2					
		3.3					
		3.4					
		3.5					
		3.6					
		3.7					
		3.8					
		3.9					
		4.0					
		4.1					
		4.2					
		4.3					
		4.4					
		4.5					
		4.6					
		4.7					
		4.8					
		4.9					
		5.0					
		5.1					
		5.2					
		5.3					
		5.4					
		5.5					
		5.6					
		5.7					
		5.8					
		5.9					
		6.0					
		6.1					
		6.2					
		6.3					
		6.4					
		6.5					
		6.6					
		6.7					
		6.8					
		6.9					
		7.0					
		7.1					
		7.2					
		7.3					
		7.4					
		7.5					
		7.6					
		7.7					
		7.8					
		7.9					
		8.0					
		8.1					
		8.2					
		8.3					
		8.4					
		8.5					
		8.6					
		8.7					
		8.8					
		8.9					
		9.0					
		9.1					
		9.2					
		9.3					
		9.4					
		9.5					
		9.6					
		9.7					
		9.8					
		9.9					
		10.0					

EXCAVATION METHOD: 50 Ø Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger





Borehole No. 85

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 of 1

Job No. 16 530

Date Excavated: 11-04-03

RL Ground:

Logged By: DTKONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Front							
Black ORGANIC CLAYEY SILT moist stiff friable	xy	0.0					
	xy	0.1					
	xy	0.2					
Mixed brown SANDY SILT slightly clayey moist very stiff	xy	0.3					
	xy	0.4	203/31				
	xy	0.5					
	xy	0.6					
	xy	0.7					
	xy	0.8					
Light grey SAND moist medium dense	xy	0.9					
Mixed brown SILT some clayey some sandy	xy	1.0	UTP				
	xy	1.1					
	xy	1.2	160/55				
	xy	1.3					
Brown SILT moist very stiff friable	xy	1.4					
Light brown SANDY SILT moist very stiff friable	xy	1.5					
Brown SILT moist very stiff friable	xy	1.6					
	xy	1.7	194/85				
	xy	1.8					
	xy	1.9					
End of Borehole		2.0					
		2.1	>209				
Rear							
Black ORGANIC CLAYEY SILT moist stiff friable	xy	0.0					
Brown SILT slightly clayey moist very stiff friable	xy	0.1					
	xy	0.2					
	xy	0.3					
Grades brown CLAYEY SILT moist very stiff	xy	0.4	129/31				
	xy	0.5					
	xy	0.6					
	xy	0.7					
	xy	0.8					
	xy	0.9	126/28				
End of Borehole		1.0					
		1.1	128/23				

EXCAVATION METHOD: 50 Ø Hand Auger

EXCAVATION METHOD: 5D  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger



EXCAVATION METHOD: 50  $\phi$  Hand Auger



Borehole No. 98

Sheet: 1 Of: 1

Logged By: DT KONTZE

[illegible]

EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger



Sheet: 1 Of: 1

Logged By: DT KONTZE

Date Excavated: 14-D4-03 RL Ground:

Logged By: DT KONTZE

EXCAVATION METHOD: 5D  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger

Borehole No. 102

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 14-04-03

RL Ground:

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Front							
Black ORGANIC CLAYEY SILT moist stiff friable	X X X X X X	0.0					
Brown SILT slightly clayey moist very stiff friable	X X X X X X	0.3					
Grades brown CLAYEY SILT moist very stiff	X X X X X X	0.6	145/58				
grades light brown	X X X X X X	0.8	91/18				
End of Borehole		1.0					
		1.1	89/48				
Rear							
Black ORGANIC CLAYEY SILT moist stiff friable	X X X X X X	0.0					
Brown SILT moist very stiff friable	X X X X X X	0.2					
grades light brown slightly clayey	X X X X X X	0.5	152/32				
	X X X X X X	0.8	155/35				
End of Borehole		1.0					
		1.1	132/23				

EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger

[illegible]

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level	Undrained Shear Strength (kPa)		
				50	100	150
<b>Upper South</b>						
Black ORGANIC CLAYEY SILT moist stiff friable	XX	0.0				
Brown SILT moist very stiff friable	XX	0.2				
	X					
	XX					
	X					
grades slightly clayey	XX	0.6	66/28			
	X					
	XX	0.8	45/17			
	X					
End of Borehole		1.0				
		1.1	158/45			
<b>Lower North</b>						
Black ORGANIC CLAYEY SILT moist stiff friable	XX	0.0				
	XX					
	XX					
Dark brown CLAYEY SILT moist hard plastic	X	0.3				
Brown SILT moist hard friable	XX	0.4	UTP			
	X					
	XX	0.6	UTP			
	X					
grades slightly clayey	XX	0.8				
	X					
	XX	0.9	220/			
	X					
End of Borehole		1.0				
		1.1	198/55			

EXCAVATION METHOD: 50  $\phi$  Hand Auger



EXCAVATION METHOD: 50  $\phi$  Hand Auger



Sheet: 1 Of: 1

Logged By: DT KONTZE

Date Excavated:	14-D4-D3	RL Ground:
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Logged By: DT KONTZE

[illegible]

EXCAVATION METHOD: 50  $\phi$  Hand Auger



Borehole No. 110

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 of 1

Job No. 16 530

Date Excavated: 30-04-03  
14-04-03

RL Ground:

Logged By: DTKONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level	Undrained Shear Strength (kPa)		
				50	100	150
Front. North						
Black ORGANIC CLAYEY SILT moist stiff friable	XX XX XX	0.0 — —				
Brown SILT moist very stiff friable	X XY X XY X XX X	0.3 — 0.5 — — 0.8 —	79/15 120/17			
End of Borehole		1.0 1.1	158/48			
Rear South						
Black ORGANIC CLAYEY SILT moist stiff friable	XX XX XX X	0.0 — — —				
Dark brown SILT moist very stiff friable	XY X XY X XY X	0.4 — — 0.7 — 0.9 —	105/28 106/32			
becomes brown slightly sandy slightly clayey		1.0 1.1	125/28			
End of Borehole						

EXCAVATION METHOD: 50 ø Hand Auger



Borehole No. 111

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 30-04-03  
14-04-03

RL Ground:

Logged By: DTKONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Front North							
Black ORGANIC CLAYEY SILT moist stiff friable	X X X X	0.0 0.1					
Mixed brown SILT some clayey some sandy very moist very stiff	X X X X X X X X	0.2 0.3 0.5 0.7	152/ 49				
Brown SILT moist very stiff friable	X X X X	0.7 0.8 1.0	160/ 72				
End of Borehole		1.1	189/ 69				
Rear South							
Black ORGANIC CLAYEY SILT moist stiff friable	X X X X X X X X	0.0 0.1 0.2 0.3					
Brown SILT slightly sandy moist very stiff friable	X X X X X X	0.4 0.5 0.7	145/ 43				
grades SILT slightly clayey not sandy	X X X X X	0.8 0.9 1.0	142/ 42				
End of Borehole		1.1	112/ 22				

EXCAVATION METHOD: 50 ø Hand Auger



Sheet: 1 Of: 1

Logged By: DT KONTZE

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Rear middle							
Black ORGANIC CLAYEY SILT moist stiff friable	XX	0.0					
	XX						
	XX						
Dark brown SILT moist very stiff friable	X	0.3					
	XX	0.4	175/66				
	X						
	XX						
	X	0.7	154/42				
Grades brown CLAYEY SILT moist very stiff plastic	XX						
	-X-						
End of Borehole		1.0					
		1.1	142/39				

EXCAVATION METHOD: 50  $\phi$  Hand Auger

Borehole No. 113

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 29-04-03

RL Ground:

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X X	0.0					
Mixed brown CLAYEY SILT moist very stiff slightly friable	X X X X X X X X X X X X X X	0.2  0.3  0.8	  166/ 48  142/ 52				
End of Borehole	X	1.0 1.1	131/ 29				

EXCAVATION METHOD: 5D  $\phi$  Hand Auger

Borehole No. 114

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 29-04-03 RL Ground:

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane	Groundwater Level	Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X	0.0					
Mixed brown SANDY SILT slightly clayey moist very stiff friable	X	0.3					
	X	0.5	160/40				
	X	0.8	148/49				
	X	0.9					
	X	1.0					
Brown SILT moist very stiff friable	X	1.1	168/55				
End of Borehole							

EXCAVATION METHOD: 50  $\phi$  Hand Auger

Borehole No. 115

Site: MAYFIELD RESIDENTIAL SUBDIVISION STAGE 2

Sheet: 1 Of: 1

Job No. 16 530

Date Excavated: 29-04-03

RL Ground:

Logged By: DT KONTZE

Description of Soil	Soil Symbol	Depth (m)	Miniature Shear Vane Groundwater Level		Undrained Shear Strength (kPa)		
					50	100	150
Black ORGANIC CLAYEY SILT moist stiff friable	X <sub>cy</sub>	0.0					
Mixed brown SILT slightly clayey slightly sandy moist very stiff friable	X <sub>x</sub>	0.2					
	X <sub>x</sub>						
	X <sub>x</sub>						
	X <sub>x</sub>	0.3	154/49				
	X <sub>x</sub>						
Brown SILT moist very stiff friable	X <sub>y</sub>	0.8	149/45				
End of Borehole	X	1.0					
		1.1	142/26				

EXCAVATION METHOD: 50  $\phi$  Hand Auger





Borehole No. 116

Sheet: 1 Of: 1

Logged By: DT KONTZE

[illegible]

EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger



Sheet: 1 Of: 1

Logged By: DT KONTZE

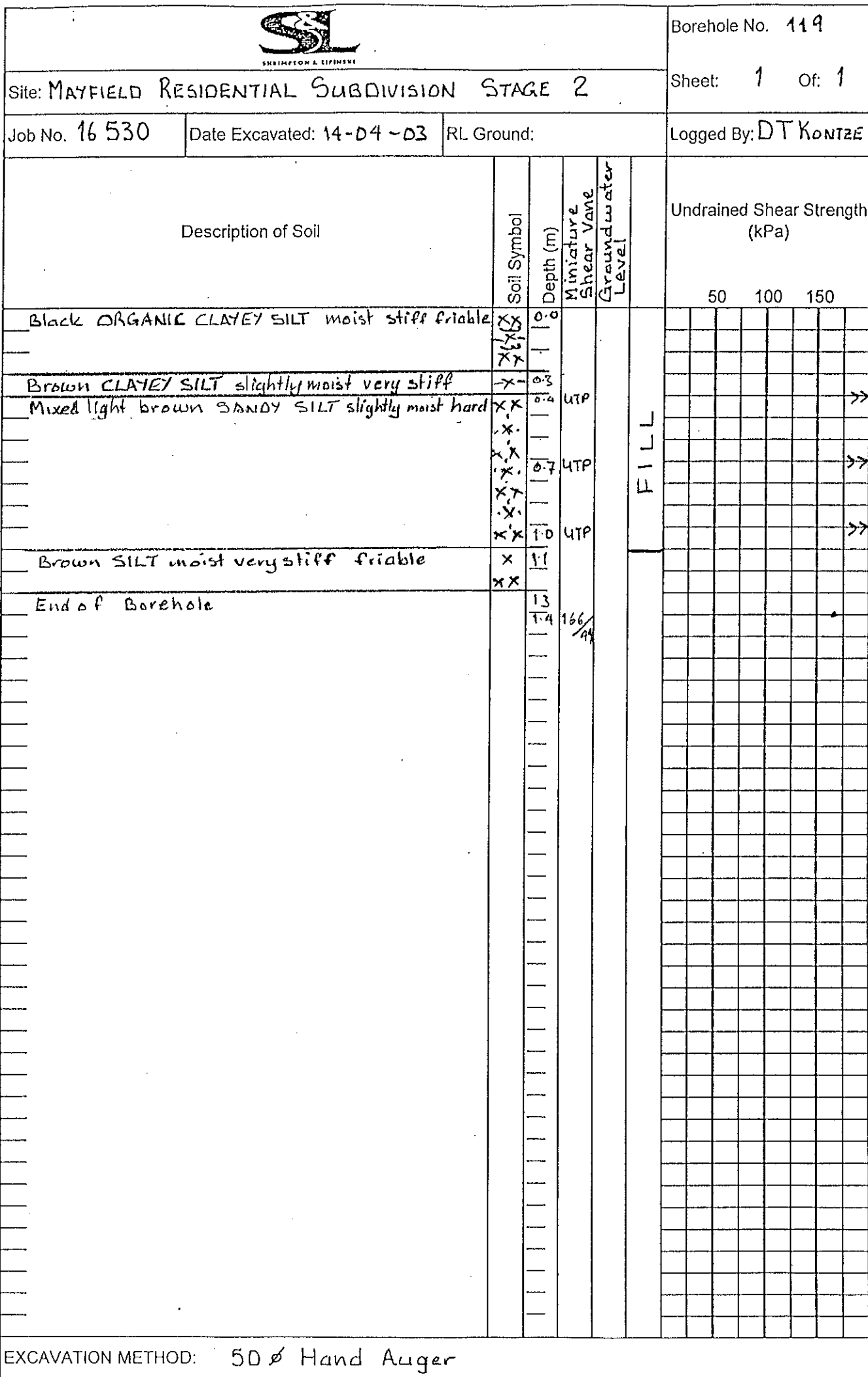
Date Excavated: 14-04-03

RL Ground:

Logged By: DT KONTZE

[illegible]

EXCAVATION METHOD: 50  $\phi$  Hand Auger





Sheet: 1 Of: 1

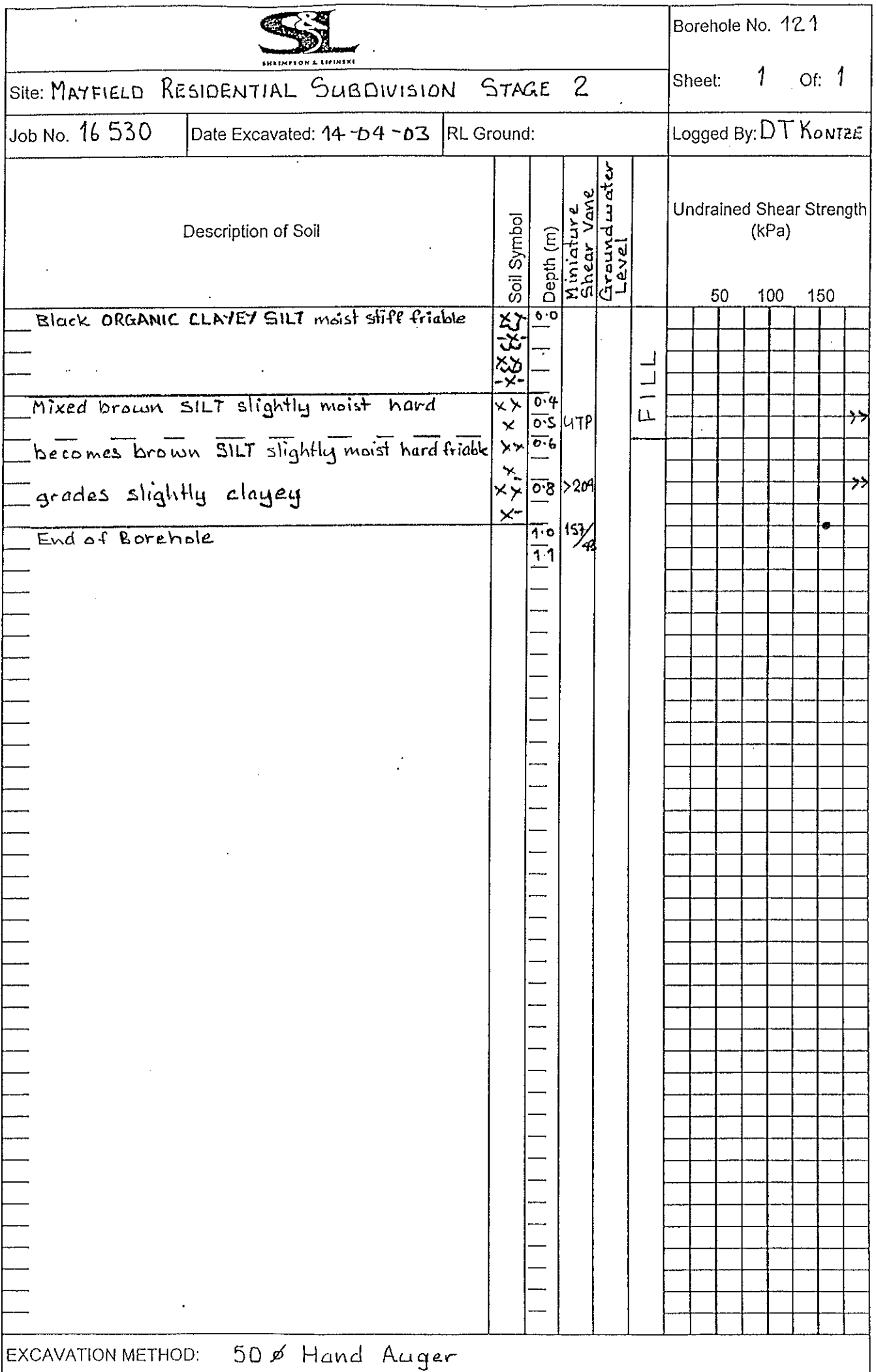
Job No. 16 530

Date Excavated: 01-05-03	RL Ground:
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Logged By: DT KONTZE

EXCAVATION METHOD: 50  $\phi$  Hand Auger

EXCAVATION METHOD: 50  $\phi$  Hand Auger





Test Pit No.

3

Site: MAYFIELD

Sheet: 1

Of: 1

Job No. 16530

Date Excavated: 19/7/02

RL Ground:

Logged By: MH

Description of Soil

Soil Symbol

Depth (m)

CALIBRATED  
VANEUndrained Shear Strength  
(kPa)

50 100 150

TOPSOIL : Hard ; Stockpile

HAUL ROAD  
LEVEL.

SILT : Brown yellow, very stiff

POST ROTORHEM ASH

DRY

EOP @ 3.6m.

EXCAVATION METHOD:

EXCAVATION METHOD:





Test Pit No. 5

Sheet: 1 Of: 1

Site: MAYFIELD

Job No. 16530

Date Excavated: 19/7/02

RL Ground:

Logged By: MH

Description of Soil

Soil Symbol

Depth (m)

Undrained Shear Strength (kPa)

50 100 150

TOPSOIL

SILT : brown yellow  
with some roots

121	115	98	120	150
50	38	40	39	62

Natural Buried topsoil

SILT : Brown yellow

FILLING

w  
w

x x

x

x x

x

x x

NATURAL ASH SOILS

DEC

105  
6098  
48156  
86180  
69175  
52180  
43

&gt;200

EXCAVATION METHOD:



Test Pit No. 6

Sheet: Of:

Site: MAYFIELD

Job No. 16530

Date Excavated: 19/7/02

RL Ground:

Logged By: MH

Description of Soil	Soil Symbol	Depth (m)	CALIBRATED VANES	Undrained Shear Strength (kPa)		
				50	100	150
TOPSOIL	X	0.0				
SILT: Brown yellow with rootlets / small twigs	X	0.5	130 55			
SILT: clayey, pale brown	X	1.0	181 65			
brown yellow, very stiff	X	1.5	153 52			
NATURAL ASH SOILS	X	2.0	156 52			
	X	2.5	182 51			
	X	3.0				
EOP @ 3.0m.						

EXCAVATION METHOD:



Test Pit No. 7

Sheet 1 Of 1

Site: MAYFIELD

Job No. 16530

Date Excavated: 19/7/02

RL Ground:

Logged By:

Description of Soil

Soil Symbol

Depth (m)

CALIBRATED  
VANIEUndrained Shear Strength  
(kPa)

50 100 150

85% Brown yellow clayey silt

10% Topsoil

5% rootlets

 $\frac{130}{40}$   $\frac{110}{39}$   $\frac{117}{40}$   $\frac{160}{50}$  $\frac{120}{38}$   $\frac{118}{38}$   $\frac{125}{38}$   $\frac{117}{30}$ 

SILT: clayey, brown yellow very stiff

FILLING

x x

x

x x

x x

x

x

x x

x

x

x x

x

x

x

x

x

x

x

x

x

x

x

NATURAL ASH SOILS

 $\frac{130}{40}$  $\frac{175}{55}$  $\frac{185}{43}$  $\frac{182}{45}$  $\frac{159}{31}$  $\frac{156}{27}$  $\frac{164}{34}$ 

EXCAVATION METHOD:

## BOREHOLE LOG

Connell Wagner

[illegible]